```
**************************************************
*                                                *
*           STAAD.Pro V8i SELECTseries4           *
*           Version  20.07.09.31                 *
*           Proprietary Program of               *
*           Bentley Systems, Inc.                *
*           Date=    DEC 22, 2014                *
*           Time=    10: 2: 1                    *
*                                                *
*      USER ID:                                  *
**************************************************
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```
     1. STAAD SPACE DXF IMPORT OF DIBUJO3.DXF
INPUT FILE: Sttad Talleres.STD
     2. START JOB INFORMATION
     3. ENGINEER DATE 17-DEC-14
     4. END JOB INFORMATION
     5. INPUT WIDTH 79
     6. UNIT METER KN
     7. JOINT COORDINATES
     8. 5 2.96444E-006 3.3 -6.73099; 6 38.19 3.3 -6.73099; 7 38.19 3.3 2.14284E-006
     9. 8 2.96444E-006 3.3 2.14284E-006; 10 7.95 3.3 -6.73099
    10. 11 7.95 3.3 2.14284E-006; 12 11.07 3.3 -6.731; 13 11.07 3.3 -8.57159E-007
    11. 14 19.07 3.3 -6.731; 15 19.07 3.3 -8.57159E-007; 16 27.07 3.3 -6.731
    12. 17 27.07 3.3 -8.57159E-007; 18 30.19 3.3 -6.731; 19 30.19 3.3 -8.57159E-007
    13. 20 2.96444E-006 -4E-016 -6.73099; 21 7.95 -4E-016 -6.73099
    14. 22 11.07 -4E-016 -6.731; 23 7.95 3.3 -3.3655; 24 7.95 -1.02E-014 -3.3655
    15. 28 11.07 -2E-014 2.14284E-006; 29 7.95 -2E-014 2.14284E-006
    16. 30 2.96444E-006 -2E-014 2.14284E-006; 31 19.07 -4E-016 -6.731
    17. 32 19.07 -1.95E-014 -8.57159E-007; 35 30.19 -4E-016 -6.731
    18. 36 27.07 -1.95E-014 -8.57159E-007; 37 30.19 -1.95E-014 -8.57159E-007
    19. 38 38.19 -4E-016 -6.73099; 41 38.19 -2E-014 2.14284E-006; 42 30.19 3.3 -3.3655
    20. 43 30.19 -1.02E-014 -3.3655; 44 27.07 3.3 -3.3655; 45 27.07 -1.02E-014 -3.3655
    21. 48 3.9 3.3 -6.73099; 49 3.9 -4E-016 -6.73099; 50 3.9 3.3 -8.57159E-007
    22. 51 3.9 -4E-016 -8.57159E-007; 52 15.12 3.3 -6.73099; 53 15.12 -4E-016 -6.73099
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    25. 59 23.07 -4E-016 -8.57159E-007; 60 34.24 3.3 -6.73099
    26. 61 34.24 -4E-016 -6.73099; 62 34.24 3.3 -8.57159E-007
    27. 63 34.24 -4E-016 -8.57159E-007; 64 2.96444E-006 3.3 -3.36549
    28. 65 2.96444E-006 -1.06E-014 -3.36549; 66 7.95 5.3 -3.3655
    29. 67 7.95 3.7 2.14284E-006; 68 7.95 5.64349 -4.088; 69 7.95 3.31358 0.812822
    30. 71 7.95 3.7 -6.731; 72 7.95 4.7 -3.3655; 73 7.95 4.7 -4.0655
    31. 74 7.95 3.10286 -8.32267; 76 2.96444E-006 5.3 -3.3655
    32. 78 2.96444E-006 3.7 2.14284E-006; 79 2.96444E-006 5.64349 -4.088
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    38. 94 11.07 3.10286 -8.32267; 106 27.07 5.3 -3.3655; 107 27.07 3.7 -8.57159E-007
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154. 566 138 306; 567 161 312; 568 162 313; 569 139 307; 570 306 108; 571 312 165
155. 572 313 166; 573 307 109; 574 117 317; 575 169 323; 576 170 324; 577 118 318
156. 578 317 127; 579 323 173; 580 324 174; 581 318 128
157. DEFINE MATERIAL START
158. ISOTROPIC CONCRETE
159. E 2.17185E+007
160. POISSON 0.17
161. DENSITY 23.5616
162. ALPHA 1E-005
163. DAMP 0.05
164. TYPE CONCRETE
165. STRENGTH FCU 21000
166. ISOTROPIC STEEL
167. E 2.05E+008
168. POISSON 0.3
169. DENSITY 76.8195
170. ALPHA 1.2E-005
171. DAMP 0.03
172. TYPE STEEL
173. STRENGTH FY 253200 FU 407800 RY 1.5 RT 1.2
174. END DEFINE MATERIAL
175. MEMBER PROPERTY AMERICAN
176. 3 5 TO 7 10 11 43 46 49 52 55 58 67 70 73 76 79 82 85 TO 88 91 TO 95 -
177. 97 TO 101 104 TO 109 112 113 115 TO 117 119 TO 121 123 TO 125 131 TO 133 -
178. 135 TO 137 139 TO 141 153 TO 155 160 163 165 TO 167 169 171 173 177 TO 204 -
179. 360 TO 366 PRIS YD 0.25 ZD 0.2
180. 12 TO 15 17 TO 21 23 TO 26 28 TO 30 32 TO 42 44 47 48 50 53 54 56 65 66 68 -
181. 71 72 74 77 78 80 114 118 122 130 134 138 152 158 161 164 168 174 TO 176 -
182. 353 TO 359 430 431 452 TO 457 PRIS YD 0.25 ZD 0.25
183. MEMBER PROPERTY EUROPEAN
184. 143 146 440 TO 451 466 TO 477 486 TO 497 506 TO 519 526 530 537 538 545 -
185. 549 TABLE FR UPN200
186. 148 151 206 208 213 214 218 221 224 227 230 233 236 239 520 TO 525 -
187. 527 TO 529 531 TO 536 539 TO 544 546 TO 548 550 TO 581 TABLE ST UPN140
188. MEMBER PROPERTY EUROPEAN
189. 396 TO 429 PRIS YD 0.25 ZD 0.25
190. 367 TO 395 PRIS YD 0.35 ZD 0.35
191. CONSTANTS
192. MATERIAL CONCRETE MEMB 3 5 TO 7 10 TO 15 17 TO 21 23 TO 26 28 TO 30 -
193. 32 TO 44 46 TO 50 52 TO 56 58 65 TO 68 70 TO 74 76 TO 80 82 85 TO 88 -
194. 91 TO 95 97 TO 101 104 TO 109 112 TO 125 130 TO 141 152 TO 155 158 160 161 -
195. 163 TO 169 171 173 TO 204 353 TO 431 452 TO 457
196. MATERIAL STEEL MEMB 143 146 148 151 206 208 213 214 218 221 224 227 230 233 -
197. 236 239 440 TO 451 466 TO 477 486 TO 497 506 TO 581
198. SUPPORTS
199. 20 TO 22 24 28 TO 32 35 TO 38 41 43 45 49 51 53 55 57 59 61 63 65 134 144 -
200. 145 TO 146 FIXED
201. DEFINE COLOMBIAN ACCIDENTAL LOAD
202. ZONE 0.16 I 1.1 S 1.6
203. SELFWEIGHT 1
204. MEMBER WEIGHT
205. 206 213 218 221 230 233 236 239 520 TO 523 527 528 531 532 535 536 539 540 -
206. 543 544 547 548 553 554 557 TO 560 563 564 567 568 571 572 575 576 579 -
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207. 580 UNI 1.9
208. 143 146 148 151 208 214 224 227 518 519 524 TO 526 529 530 533 534 537 538 -
209. 541 542 545 546 549 TO 552 555 556 561 562 565 566 569 570 573 574 577 578 -
210. 581 UNI 0.5
211. 396 TO 409 UNI 6.58
212. 410 TO 419 422 427 UNI 3.29
213. LOAD 1 LOADTYPE NONE  TITLE SISMO X
214. COLOMBIAN LOAD X 1 ACC 1
215. LOAD 2 LOADTYPE NONE  TITLE SISMO Z
216. COLOMBIAN LOAD Z 1 ACC 1
217. LOAD 3 CARGA VIVA
218. MEMBER LOAD
219. 206 213 218 221 230 233 236 239 520 TO 523 527 528 531 532 535 536 539 540 -
220. 543 544 547 548 553 554 557 TO 560 563 564 567 568 571 572 575 576 579 -
221. 580 UNI GY -1.19
222. 143 146 148 151 208 214 224 227 518 519 524 TO 526 529 530 533 534 537 538 -
223. 541 542 545 546 549 TO 552 555 556 561 562 565 566 569 570 573 574 577 578 -
224. 581 UNI GY -0.3
225. LOAD 4 CARGA MUERTA
226. SELFWEIGHT Y -1
227. MEMBER LOAD
228. 95 107 TO 109 112 113 154 LIN Y -4.4 -1.1
229. 6 7 10 11 153 155 173 LIN Y -1.1 -4.4
230. 206 213 218 221 230 233 236 239 520 TO 523 527 528 531 532 535 536 539 540 -
231. 543 544 547 548 553 554 557 TO 560 563 564 567 568 571 572 575 576 579 -
232. 580 UNI GY -1.9
233. 143 146 148 151 208 214 224 227 518 519 524 TO 526 529 530 533 534 537 538 -
234. 541 542 545 546 549 TO 552 555 556 561 562 565 566 569 570 573 574 577 578 -
235. 581 UNI GY -0.5
236. 396 TO 409 UNI GY -6.58
237. 410 TO 419 422 427 UNI GY -3.29
238. LOAD 56 LOADTYPE NONE  TITLE CARGA DE VIENTO
239. JOINT LOAD
240. 73 74 84 85 93 94 113 114 122 123 132 133 142 143 147 148 151 152 155 156 -
241. 159 160 163 164 167 168 171 172 280 TO 283 297 TO 300 308 TO 311 319 TO 321 -
242. 322 FY -1.42 FZ 0.66
243. 68 69 79 80 89 90 108 109 117 118 127 128 138 139 149 150 153 154 157 158 -
244. 161 162 165 166 169 170 173 174 278 279 284 285 295 296 301 302 306 307 312 -
245. 313 317 318 323 324 FY 1.42 FZ 0.66
246. LOAD COMB 5 1.2 CM + 1.6 CV
247. 4 1.2 3 1.6
248. LOAD COMB 6 1.2 CM + CV + (SX/R + 0,30 SZ/R)
249. 4 1.2 3 1.0 1 0.255 2 0.0765
250. LOAD COMB 7 1.2 CM + CV - (SX/R - 0,30 SZ/R)
251. 4 1.2 3 1.0 1 -0.255 2 0.0765
252. LOAD COMB 8 1.2 CM + CV + (SX/R - 0,30 SZ/R)
253. 4 1.2 3 1.0 1 0.255 2 -0.0765
254. LOAD COMB 9 1.2 CM + CV - (SX/R + 0,30 SZ/R)
255. 4 1.2 3 1.0 1 -0.255 2 -0.0765
256. LOAD COMB 10 1.2 CM + CV + (0.3 SX/R + SZ/R)
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257. 4 1.2 3 1.0 1 0.0765 2 0.255
258. LOAD COMB 11 1.2 CM + CV - (0.3 SX/R - SZ/R)
259. 4 1.2 3 1.0 1 -0.0765 2 0.255
260. LOAD COMB 12 1.2 CM + CV + (0.3 SX/R - SZ/R)
261. 4 1.2 3 1.0 1 0.0765 2 -0.255
262. LOAD COMB 13 1.2 CM + CV - (0.3 SX/R + SZ/R)
263. 4 1.2 3 1.0 1 -0.0765 2 -0.255
264. LOAD COMB 14 0.9 CM + (SX/R + 0,30 SZ/R)
265. 4 0.9 1 0.255 2 0.0765
266. LOAD COMB 15 0.9 CM - (SX/R - 0,30 SZ/R)
267. 4 0.9 1 -0.255 2 0.0765
268. LOAD COMB 16 0.9 CM + (SX/R - 0,30 SZ/R)
269. 4 0.9 1 0.255 2 -0.0765
270. LOAD COMB 17 0.9 CM - (SX/R + 0,30 SZ/R)
271. 4 0.9 1 -0.255 2 -0.0765
272. LOAD COMB 18 0.9 CM + (0.3 SX/R + SZ/R)
273. 4 0.9 1 0.0765 2 0.255
274. LOAD COMB 19 0.9 CM - (0.3 SX/R - SZ/R)
275. 4 0.9 1 -0.0765 2 0.255
276. LOAD COMB 20 0.9 CM + (0.3 SX/R - SZ/R)
277. 4 0.9 1 0.0765 2 -0.255
278. LOAD COMB 21 0.9 CM - (0.3 SX/R + SZ/R)
279. 4 0.9 1 -0.0765 2 -0.255
280. *COMBINACIONES PARA DISEÑO DE CIMENTACION
281. *COMBINACIONES DE SERVICIO
282. LOAD COMB 22 CM + CV
283. 4 1.0 3 1.0
284. LOAD COMB 23 CM + 0,75 CV + 0.75 (0.7)(SX + 0,3 SZ)/R
285. 4 1.0 3 0.75 1 0.134 2 0.04
286. LOAD COMB 24 CM + 0,75 CV - 0.75 (0.7)(SX - 0,3 SZ)/R
287. 4 1.0 3 0.75 1 -0.134 2 0.04
288. LOAD COMB 25 CM + 0,75 CV + 0.75 (0.7)(SX - 0,3 SZ)/R
289. 4 1.0 3 0.75 1 0.134 2 -0.04
290. LOAD COMB 26 CM + 0,75 CV - 0.75 (0.7)(SX + 0,3 SZ)/R
291. 4 1.0 3 0.75 1 -0.134 2 -0.04
292. LOAD COMB 27 CM + 0,75 CV + 0.75 (0.7)(0,3 SX + SZ)/R
293. 4 1.0 3 0.75 1 0.04 2 0.134
294. LOAD COMB 28 CM + 0,75 CV - 0.75 (0.7)(0.3 SX - SZ)/R
295. 4 1.0 3 0.75 1 -0.04 2 0.134
296. LOAD COMB 29 CM + 0,75 CV + 0.75 (0.7)(0.3 SX - SZ)/R
297. 4 1.0 3 0.75 1 0.04 2 -0.134
298. LOAD COMB 30 CM + 0,75 CV - 0.75 (0.7)(0.3 SX + SZ)/R
299. 4 1.0 3 0.75 1 -0.04 2 -0.134
300. LOAD COMB 31 0.6 CM + 0.700 (SX + 0.3 SZ)/R
301. 4 0.6 1 0.178 2 0.053
302. LOAD COMB 32 0.6 CM - 0.700 (SX - 0.3 SZ)/R
303. 4 0.6 1 -0.178 2 0.053
304. LOAD COMB 33 0.6 CM + 0.700 (SX - 0.3 SZ)/R
305. 4 0.6 1 0.178 2 -0.053
306. LOAD COMB 34 0.6 CM - 0.700 (SX + 0.3 SZ)/R
307. 4 0.6 1 -0.178 2 -0.053
308. LOAD COMB 35 0.6 CM + 0.700 (0.3 SX + SZ)/R
309. 4 0.6 1 0.053 2 0.178
310. LOAD COMB 36 0.6 CM - 0.700 (0.3 SX - SZ)/R
311. 4 0.6 1 -0.053 2 0.178
312. LOAD COMB 37 0.6 CM - 0.700 (0.3 SX + SZ)/R

```
313. 4 0.6 1 -0.053 2 -0.178
314. LOAD COMB 38 0.6 CM + 0.700 ( 0.3 SX - SZ)/R
315. 4 0.6 1 0.053 2 -0.178
316. *COMBINACIONES MAYORADAS
317. LOAD COMB 39 1.2 CM + 1.6 CV
318. 4 1.2 3 1.6
319. LOAD COMB 40 1.2 CM + CV + (SX/R + 0,30 SZ/R)
320. 4 1.2 3 1.0 1 0.255 2 0.0765
321. LOAD COMB 41 1.2 CM + CV - (SX/R - 0,30 SZ/R)
322. 4 1.2 3 1.0 1 -0.255 2 0.0765
323. LOAD COMB 42 1.2 CM + CV + (SX/R - 0,30 SZ/R)
324. 4 1.2 3 1.0 1 0.255 2 -0.0765
325. LOAD COMB 43 1.2 CM + CV - (SX/R + 0,30 SZ/R)
326. 4 1.2 3 1.0 1 -0.255 2 -0.0765
327. LOAD COMB 44 1.2 CM + CV + (0.3 SX/R + SZ/R)
328. 4 1.2 3 1.0 1 0.0765 2 0.255
329. LOAD COMB 45 1.2 CM + CV - (0.3 SX/R - SZ/R)
330. 4 1.2 3 1.0 1 -0.0765 2 0.255
331. LOAD COMB 46 1.2 CM + CV + (0.3 SX/R - SZ/R)
332. 4 1.2 3 1.0 1 0.0765 2 -0.255
333. LOAD COMB 47 1.2 CM + CV - (0.3 SX/R + SZ/R)
334. 4 1.2 3 1.0 1 -0.0765 2 -0.255
335. LOAD COMB 48 0.9 CM + (SX/R + 0,30 SZ/R)
336. 4 0.9 1 0.255 2 0.0765
337. LOAD COMB 49 0.9 CM - (SX/R - 0,30 SZ/R)
338. 4 0.9 1 -0.255 2 0.0765
339. LOAD COMB 50 0.9 CM + (SX/R - 0,30 SZ/R)
340. 4 0.9 1 0.255 2 -0.0765
341. LOAD COMB 51 0.9 CM - (SX/R + 0,30 SZ/R)
342. 4 0.9 1 -0.255 2 -0.0765
343. LOAD COMB 52 0.9 CM + (0.3 SX/R + SZ/R)
344. 4 0.9 1 0.0765 2 0.255
345. LOAD COMB 53 0.9 CM - (0.3 SX/R - SZ/R)
346. 4 0.9 1 -0.0765 2 0.255
347. LOAD COMB 54 0.9 CM + (0.3 SX/R - SZ/R)
348. 4 0.9 1 0.0765 2 -0.255
349. LOAD COMB 55 0.9 CM - (0.3 SX/R + SZ/R)
350. 4 0.9 1 -0.0765 2 -0.255
351. LOAD COMB 57 1.2 CM + W + CV
352. 4 1.2 56 1.0 3 1.0
353. LOAD COMB 58 1.2 CM - W + CV
354. 4 1.2 56 -1.0 3 1.0
355. LOAD COMB 59 0.9 CM + W
356. 4 0.9 56 1.0
357. LOAD COMB 60 0.9 CM - W
358. 4 0.9 56 -1.0
359. PERFORM ANALYSIS
```

P R O B L E M S T A T I S T I C S

NUMBER OF JOINTS	230	NUMBER OF MEMBERS	367
NUMBER OF PLATES	0	NUMBER OF SOLIDS	0
NUMBER OF SURFACES	0	NUMBER OF SUPPORTS	29

SOLVER USED IS THE OUT-OF-CORE BASIC SOLVER

ORIGINAL/FINAL BAND-WIDTH= 155/ 21/ 120 DOF
TOTAL PRIMARY LOAD CASES = 5, TOTAL DEGREES OF FREEDOM = 1206
SIZE OF STIFFNESS MATRIX = 145 DOUBLE KILO-WORDS
REQRD/AVAIL. DISK SPACE = 14.2/ 124502.6 MB

**WARNING: IF THIS UBC/IBC ANALYSIS HAS TENSION/COMPRESSION
OR REPEAT LOAD OR RE-ANALYSIS OR SELECT OPTIMIZE, THEN EACH
UBC/IBC CASE SHOULD BE FOLLOWED BY PERFORM ANALYSIS _CHANGE.

****
* *
* COLOMBIAN SEISMIC LOAD : *
* *
* TIME PERIODS FOR X DIRECTION: *
* Ta = 0.312 Tb = 0.294 Tuser = 0.000 *
* TIME PERIOD USED (T) = 0.312 *
* LOAD FACTOR = 1.000 *
* DESIGN BASE SHEAR = 0.440 X 1508.22 = 663.62 KN *
* *
****

****
* *
* COLOMBIAN SEISMIC LOAD : *
* *
* TIME PERIODS FOR Z DIRECTION: *
* Ta = 0.312 Tb = 0.263 Tuser = 0.000 *
* TIME PERIOD USED (T) = 0.312 *
* LOAD FACTOR = 1.000 *
* DESIGN BASE SHEAR = 0.440 X 1508.22 = 663.62 KN *
* *
****

360. LOAD LIST 1 2
361. PRINT STORY DRIFT 0.010000

STORY	HEIGHT (METE)	LOAD	AVG. DISP(CM) X	AVG. DISP(CM) Z	DRIFT(CM) X	DRIFT(CM) Z	RATIO	STATUS
BASE=	**0.00**					**ALLOW. DRIFT = L / 100**		
1	0.00	1	0.0000	0.0000	0.0000	0.0000	L /999999	PASS
		2	0.0000	0.0000	0.0000	0.0000	L /999999	PASS
2	0.30	1	0.0069	0.0000	0.0069	0.0000	L / 4318	PASS
		2	0.0000	0.0070	0.0000	0.0070	L / 4273	PASS
3	3.10	1	1.1731	0.0001	1.1662	0.0001	L / 240	PASS
		2	0.9432	1.2220	0.9432	1.2150	L / 230	PASS
4	3.30	1	1.2083	0.0000	0.0352	0.0001	L / 559	PASS
		2	-0.1582	1.2160	1.1014	0.0061	L / 18	*FAIL
5	3.63	1	1.3946	0.0001	0.1863	0.0001	L / 177	PASS
		2	0.5269	1.3642	0.6851	0.1482	L / 48	*FAIL
6	3.70	1	1.3253	0.0000	0.0693	0.0001	L / 101	PASS
		2	-0.0015	1.3641	0.5284	0.0001	L / 13	*FAIL
7	4.18	1	1.7746	0.0000	0.4493	0.0000	L / 107	PASS
		2	-0.0216	1.4391	0.0201	0.0750	L / 645	PASS
8	4.70	1	2.2896	0.0000	0.5151	0.0000	L / 100	*FAIL
		2	0.1821	1.3469	0.2037	0.0922	L / 253	PASS
9	4.91	1	2.2694	0.0000	0.0203	0.0000	L / 1054	PASS

		2	-0.0411	1.3997	0.2232	0.0528 L /	96	*FAIL
10	4.95	1	2.3724	0.0001	0.1030	0.0000 L /	35	*FAIL
		2	0.1465	1.3861	0.1876	0.0137 L /	19	*FAIL
11	5.10	1	2.4076	0.0002	0.0352	0.0002 L /	438	PASS
		2	0.0890	1.6087	0.0575	0.2227 L /	69	*FAIL
12	5.30	1	2.5210	0.0001	0.1134	0.0002 L /	172	PASS
		2	0.1884	1.3573	0.0994	0.2515 L /	78	*FAIL
13	5.64	1	2.7234	0.0001	0.2025	0.0000 L /	169	PASS
		2	0.4312	1.3291	0.2428	0.0282 L /	141	PASS

```
362. LOAD LIST 5 TO 21 57 TO 60
363. START CONCRETE DESIGN
364. CODE ACI
365. CLB 0.04 MEMB 3 5 TO 7 10 TO 15 17 TO 21 23 TO 26 28 TO 30 32 TO 44 -
366. 46 TO 50 52 TO 56 58 65 TO 68 70 TO 74 76 TO 80 82 85 TO 88 91 TO 95 -
367. 97 TO 101 104 TO 109 112 TO 125 130 TO 141 152 TO 155 158 160 161 -
368. 163 TO 169 171 173 TO 204 353 TO 395 430 431 440 TO 450 452 TO 457 -
369. 466 TO 476 486 TO 496 506 TO 516
370. CLS 0.04 MEMB 3 5 TO 7 10 TO 15 17 TO 21 23 TO 26 28 TO 30 32 TO 44 -
371. 46 TO 50 52 TO 56 58 65 TO 68 70 TO 74 76 TO 80 82 85 TO 88 91 TO 95 -
372. 97 TO 101 104 TO 109 112 TO 125 130 TO 141 152 TO 155 158 160 161 -
373. 163 TO 169 171 173 TO 204 353 TO 395 430 431 440 TO 450 452 TO 457 -
374. 466 TO 476 486 TO 496 506 TO 516
375. CLT 0.04 MEMB 3 5 TO 7 10 TO 15 17 TO 21 23 TO 26 28 TO 30 32 TO 44 -
376. 46 TO 50 52 TO 56 58 65 TO 68 70 TO 74 76 TO 80 82 85 TO 88 91 TO 95 -
377. 97 TO 101 104 TO 109 112 TO 125 130 TO 141 152 TO 155 158 160 161 -
378. 163 TO 169 171 173 TO 204 353 TO 395 430 431 440 TO 450 452 TO 457 -
379. 466 TO 476 486 TO 496 506 TO 516
380. FC 21000 MEMB 3 5 TO 7 10 TO 15 17 TO 21 23 TO 26 28 TO 30 32 TO 44 -
381. 46 TO 50 52 TO 56 58 65 TO 68 70 TO 74 76 TO 80 82 85 TO 88 91 TO 95 -
382. 97 TO 101 104 TO 109 112 TO 125 130 TO 141 152 TO 155 158 160 161 -
383. 163 TO 169 171 173 TO 204 353 TO 395 430 431 440 TO 450 452 TO 457 -
384. 466 TO 476 486 TO 496 506 TO 516
385. FYMAIN 420000 MEMB 3 5 TO 7 10 TO 15 17 TO 21 23 TO 26 28 TO 30 32 TO 44 46 -
386. 47 TO 50 52 TO 56 58 65 TO 68 70 TO 74 76 TO 80 82 85 TO 88 91 TO 95 -
387. 97 TO 101 104 TO 109 112 TO 125 130 TO 141 152 TO 155 158 160 161 -
388. 163 TO 169 171 173 TO 204 353 TO 395 430 431 440 TO 450 452 TO 457 -
389. 466 TO 476 486 TO 496 506 TO 516
390. FYSEC 420000 MEMB 3 5 TO 7 10 TO 15 17 TO 21 23 TO 26 28 TO 30 32 TO 44 46 -
391. 47 TO 50 52 TO 56 58 65 TO 68 70 TO 74 76 TO 80 82 85 TO 88 91 TO 95 -
392. 97 TO 101 104 TO 109 112 TO 125 130 TO 141 152 TO 155 158 160 161 -
393. 163 TO 169 171 173 TO 204 353 TO 395 430 431 440 TO 450 452 TO 457 -
394. 466 TO 476 486 TO 496 506 TO 516
395. DESIGN BEAM 3 5 TO 7 10 11 43 46 49 52 55 58 67 70 73 76 79 82 85 TO 88 91 -
396. 92 TO 95 97 TO 101 104 TO 109 112 113 115 TO 117 119 TO 121 123 TO 125 131 -
397. 132 TO 133 135 TO 137 139 TO 141 153 TO 155 160 163 165 TO 167 169 171 173 -
398. 177 TO 204 360 TO 366 440 TO 450 466 TO 476 486 TO 496 506 TO 516
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BEAM NO. 3 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

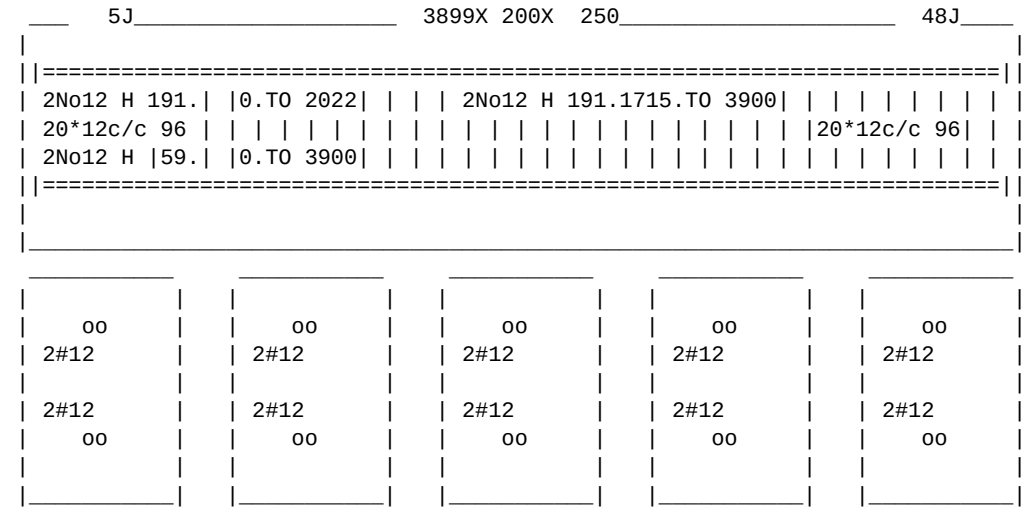
LEN - 3900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3900.	YES	YES
2	191.	2 - 12MM	0.	2022.	YES	NO
3	191.	2 - 12MM	1715.	3900.	NO	YES

B E A M N O. 3 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 1.77 KNS Vc= 27.85 KNS Vs= 0.00 KNS
Tu= 0.20 KN-MET Tc= 0.7 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1765. MM

AT END SUPPORT - Vu= 3.23 KNS Vc= 27.85 KNS Vs= 0.00 KNS
Tu= 0.20 KN-MET Tc= 0.7 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1765. MM



BEAM NO. 5 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

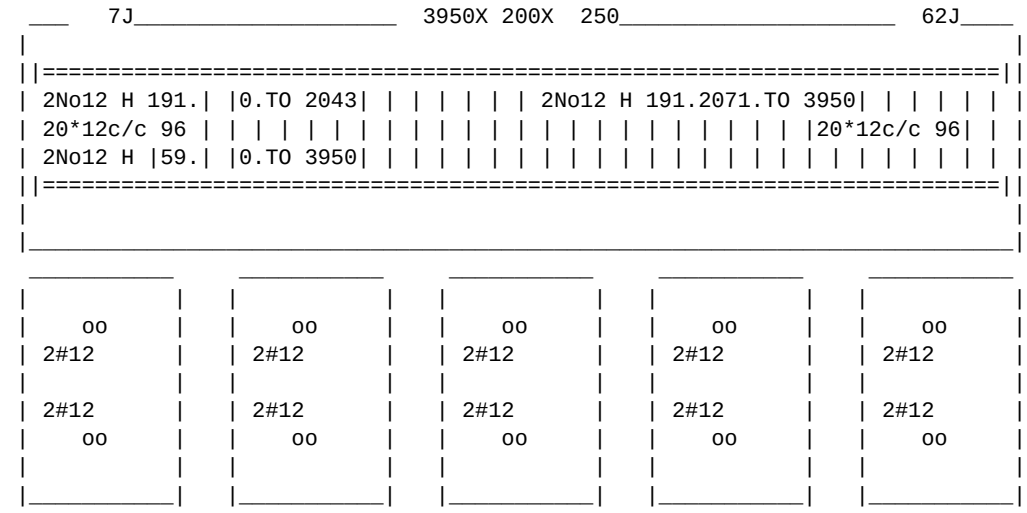
LEN - 3950. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3950.	YES	YES
2	191.	2 - 12MM	0.	2043.	YES	NO
3	191.	2 - 12MM	2071.	3950.	NO	YES

B E A M N O. 5 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.02 KNS Vc= 28.84 KNS Vs= 0.00 KNS
Tu= 0.03 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1790. MM

AT END SUPPORT - Vu= 3.04 KNS Vc= 28.84 KNS Vs= 0.00 KNS
Tu= 0.03 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1790. MM



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BEAM NO. 6 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

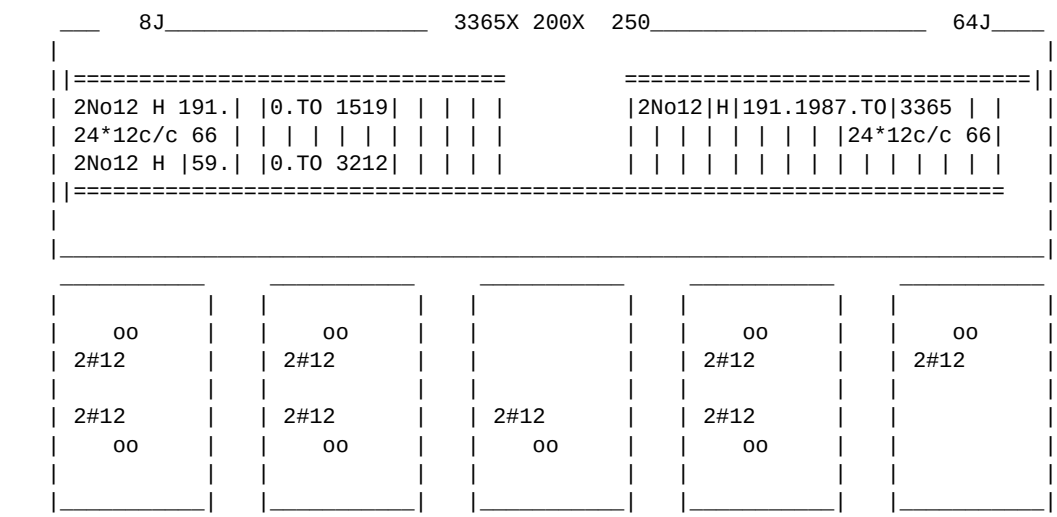
LEN - 3365. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3212.	YES	NO
2	191.	2 - 12MM	0.	1519.	YES	NO
3	191.	2 - 12MM	1987.	3365.	NO	YES

B E A M N O. 6 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 4.57 KNS Vc= 28.08 KNS Vs= 0.00 KNS
Tu= 1.07 KN-MET Tc= 0.8 KN-MET Ts= 1.4 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.63 SQ.CM.

AT END SUPPORT - Vu= 9.55 KNS Vc= 28.08 KNS Vs= 0.00 KNS
Tu= 1.07 KN-MET Tc= 0.8 KN-MET Ts= 1.4 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.63 SQ.CM.



BEAM NO. 7 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

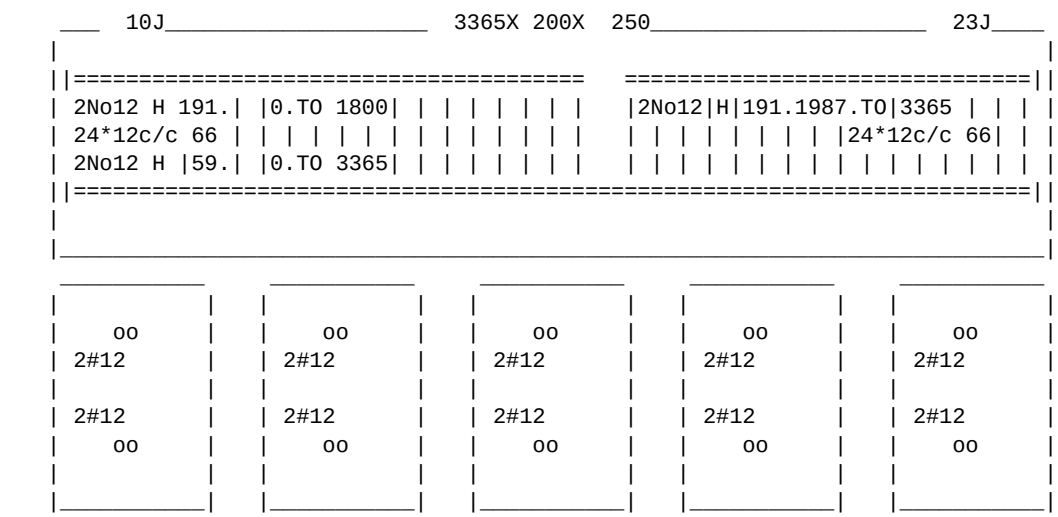
LEN - 3365. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3365.	YES	YES
2	191.	2 - 12MM	0.	1800.	YES	NO
3	191.	2 - 12MM	1987.	3365.	NO	YES

B E A M N O. 7 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 7.23 KNS Vc= 27.39 KNS Vs= 0.00 KNS
Tu= 1.03 KN-MET Tc= 0.7 KN-MET Ts= 1.4 KN-MET LOAD 8
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.60 SQ.CM.

AT END SUPPORT - Vu= 6.90 KNS Vc= 27.39 KNS Vs= 0.00 KNS
Tu= 1.03 KN-MET Tc= 0.7 KN-MET Ts= 1.4 KN-MET LOAD 8
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.60 SQ.CM.



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BEAM NO. 10 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

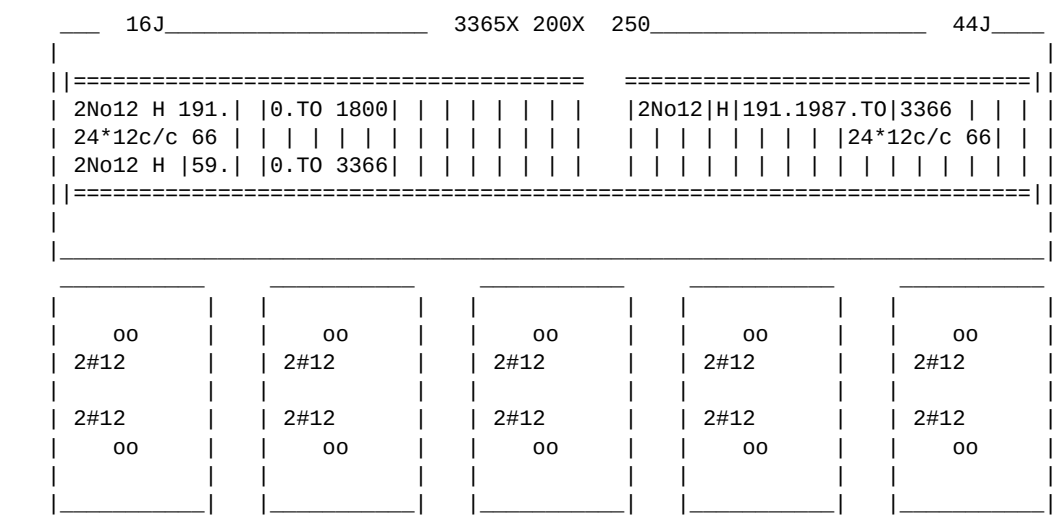
LEN - 3366. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3366.	YES	YES
2	191.	2 - 12MM	0.	1800.	YES	NO
3	191.	2 - 12MM	1987.	3366.	NO	YES

B E A M N O. 10 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 7.28 KNS Vc= 27.41 KNS Vs= 0.00 KNS
Tu= 1.03 KN-MET Tc= 0.7 KN-MET Ts= 1.4 KN-MET LOAD 8
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.60 SQ.CM.

AT END SUPPORT - Vu= 6.84 KNS Vc= 27.41 KNS Vs= 0.00 KNS
Tu= 1.03 KN-MET Tc= 0.7 KN-MET Ts= 1.4 KN-MET LOAD 8
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.60 SQ.CM.



BEAM NO. 11 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

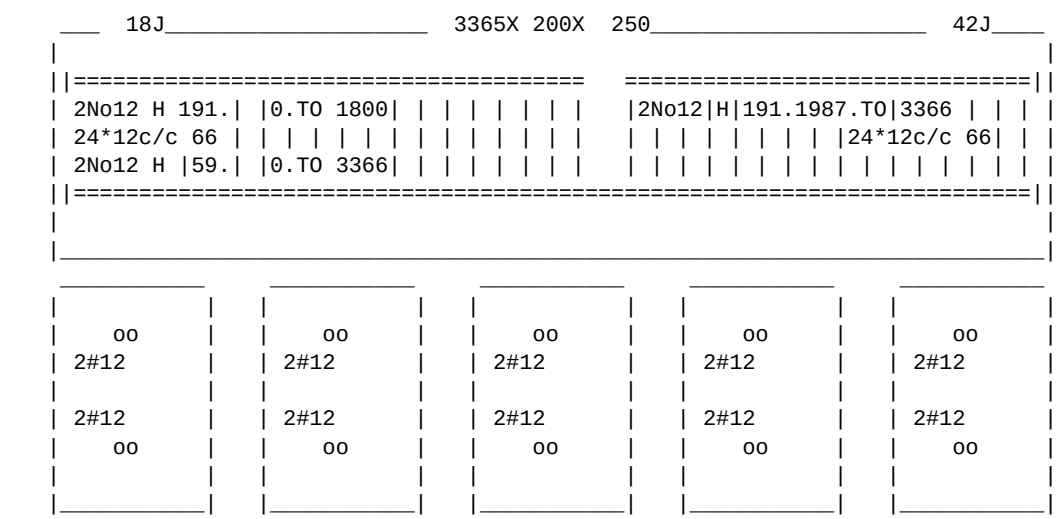
LEN - 3366. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3366.	YES	YES
2	191.	2 - 12MM	0.	1800.	YES	NO
3	191.	2 - 12MM	1987.	3366.	NO	YES

B E A M N O. 11 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 5.31 KNS Vc= 27.75 KNS Vs= 0.00 KNS
Tu= 1.03 KN-MET Tc= 0.7 KN-MET Ts= 1.4 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.60 SQ.CM.

AT END SUPPORT - Vu= 8.81 KNS Vc= 27.75 KNS Vs= 0.00 KNS
Tu= 1.03 KN-MET Tc= 0.7 KN-MET Ts= 1.4 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.60 SQ.CM.



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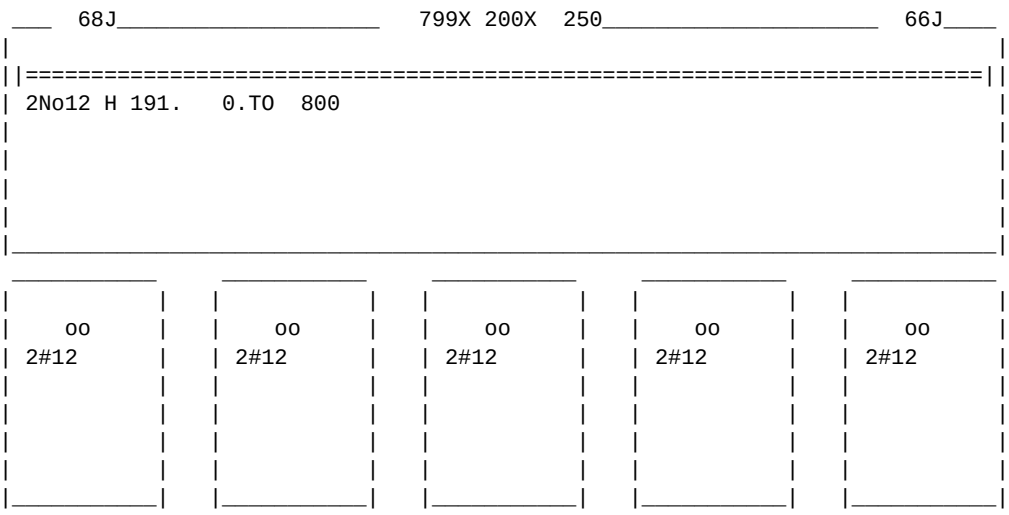
BEAM NO. 43 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 800. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	800.	YES	YES

B E A M N O. 43 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 3.85 KNS Tu= 3.5 KN-MET
Vc= 27.8 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 6 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 4.40 KNS Tu= 3.5 KN-MET
Vc= 27.8 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 6 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



BEAM NO. 46 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

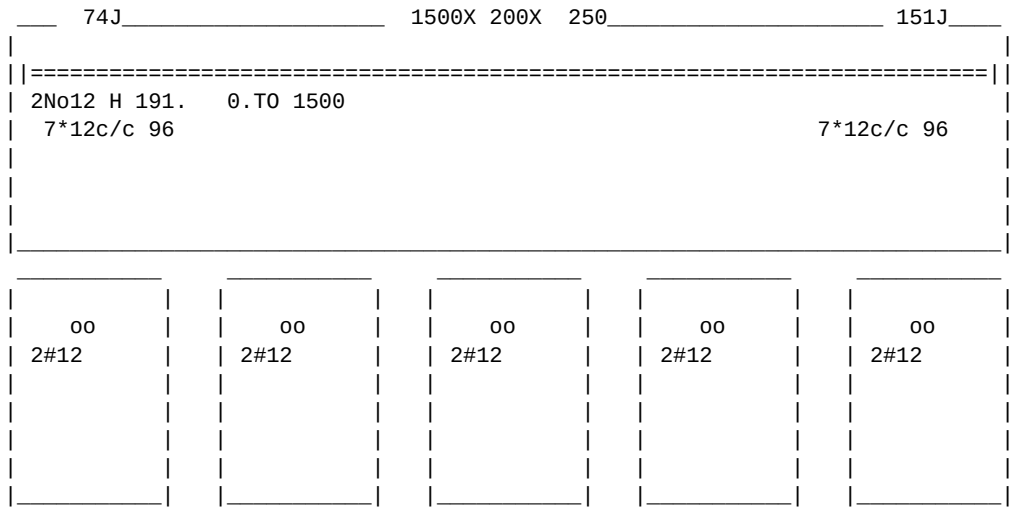
LEN - 1500. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	1500.	YES	YES

B E A M N O. 46 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 4.18 KNS Vc= 28.94 KNS Vs= 0.00 KNS
Tu= 0.11 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 565. MM

AT END SUPPORT - Vu= 5.68 KNS Vc= 28.85 KNS Vs= 0.00 KNS
Tu= 0.11 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 565. MM



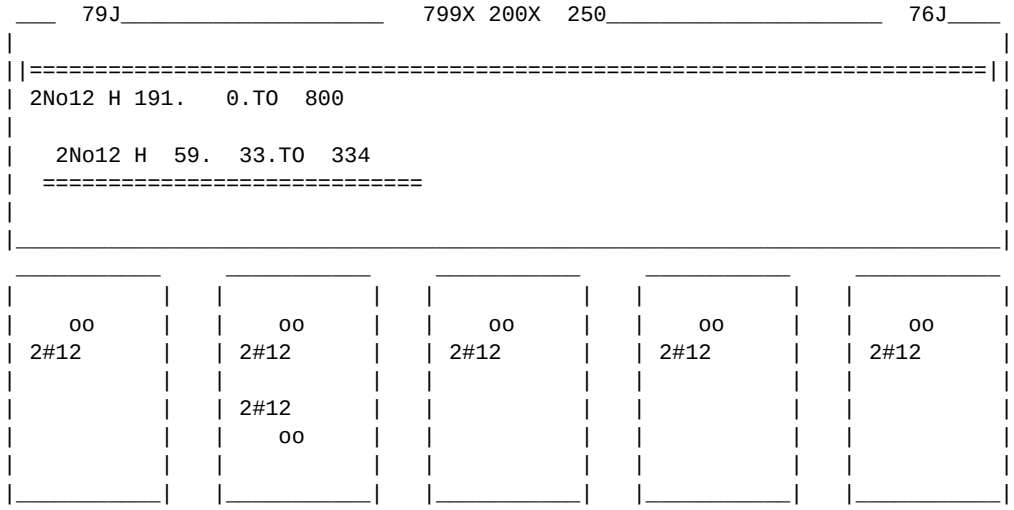
BEAM NO. 49 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 800. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	33.	334.	NO	NO
2	191.	2 - 12MM	0.	800.	YES	YES

B E A M N O. 49 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 3.31 KNS Tu= 2.6 KN-MET
Vc= 27.8 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 3.86 KNS Tu= 2.6 KN-MET
Vc= 27.8 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



BEAM NO. 52 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

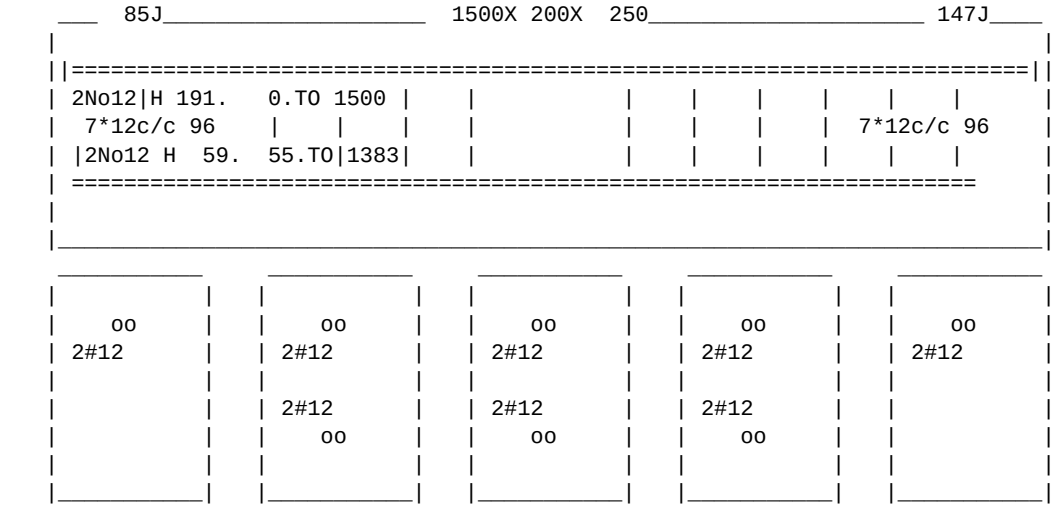
LEN - 1500. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	55.	1383.	NO	NO
2	191.	2 - 12MM	0.	1500.	YES	YES

B E A M N O. 52 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.06 KNS Vc= 29.08 KNS Vs= 0.00 KNS
Tu= 0.23 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 565. MM

AT END SUPPORT - Vu= 3.55 KNS Vc= 28.98 KNS Vs= 0.00 KNS
Tu= 0.23 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 565. MM



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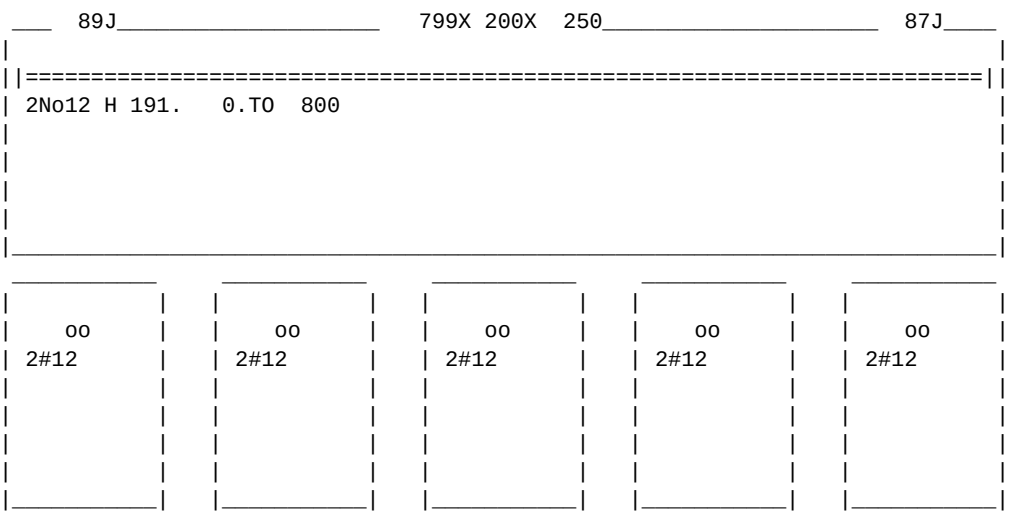
BEAM NO. 55 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 800. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	800.	YES	YES

B E A M N O. 55 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 3.99 KNS Tu= 2.8 KN-MET
Vc= 27.8 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 7 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 4.54 KNS Tu= 2.8 KN-MET
Vc= 27.8 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 7 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



BEAM NO. 58 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

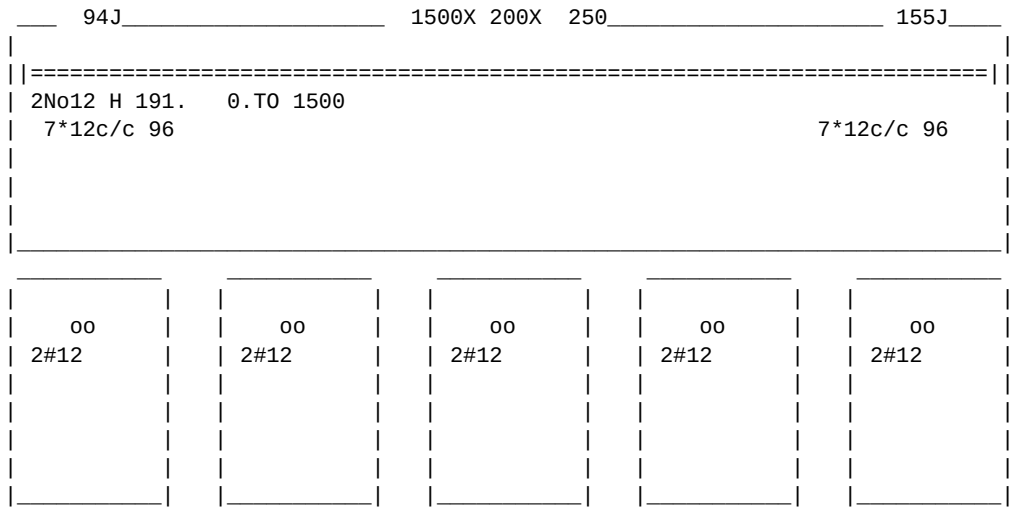
LEN - 1500. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	1500.	YES	YES

B E A M N O. 58 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 4.26 KNS Vc= 28.94 KNS Vs= 0.00 KNS
Tu= 0.10 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 565. MM

AT END SUPPORT - Vu= 5.76 KNS Vc= 28.85 KNS Vs= 0.00 KNS
Tu= 0.10 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 565. MM



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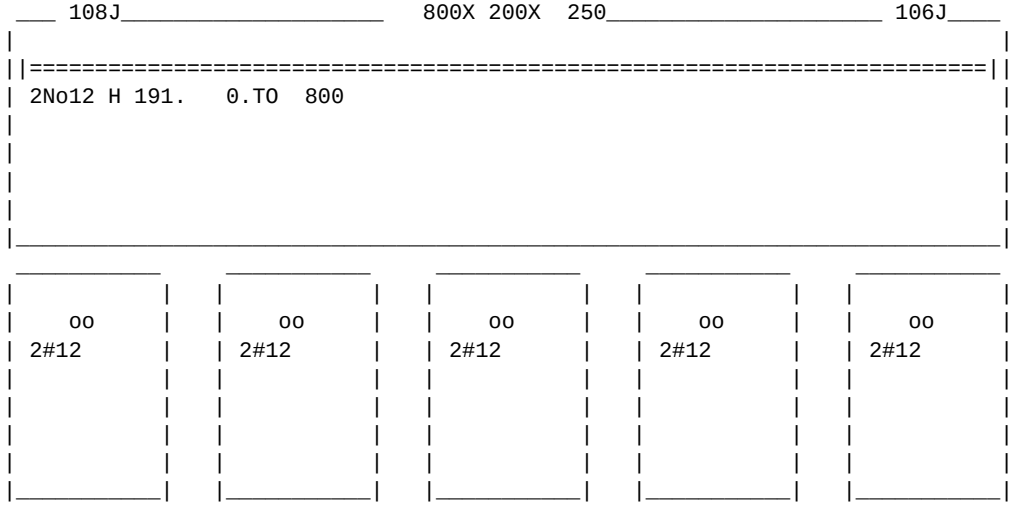
BEAM NO. 67 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 800. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	800.	YES	YES

B E A M N O. 67 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 3.85 KNS Tu= 3.2 KN-MET
Vc= 27.8 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 6 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 4.40 KNS Tu= 3.2 KN-MET
Vc= 27.8 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 6 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



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BEAM NO. 70 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

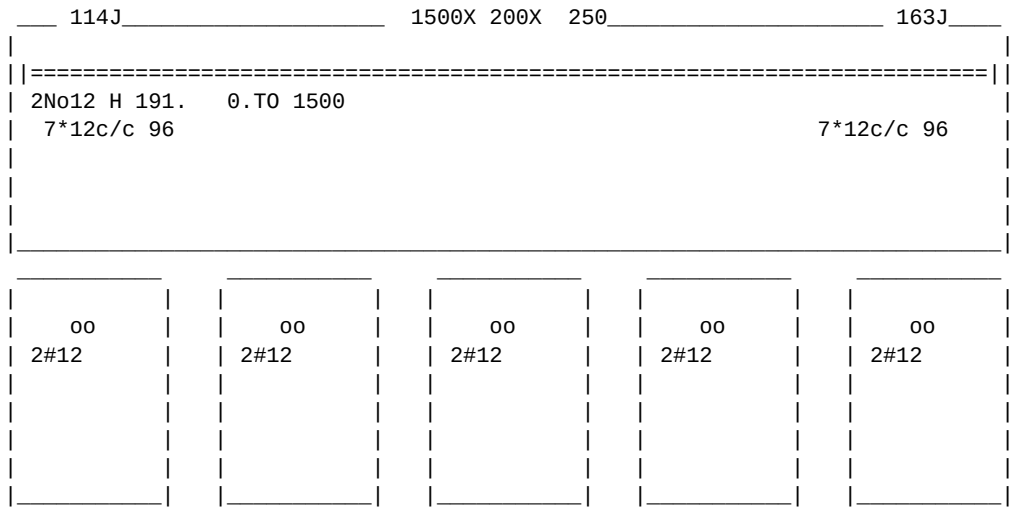
LEN - 1500. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	1500.	YES	YES

B E A M N O. 70 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 4.21 KNS Vc= 28.94 KNS Vs= 0.00 KNS
Tu= 0.12 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 565. MM

AT END SUPPORT - Vu= 5.70 KNS Vc= 28.85 KNS Vs= 0.00 KNS
Tu= 0.12 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 565. MM



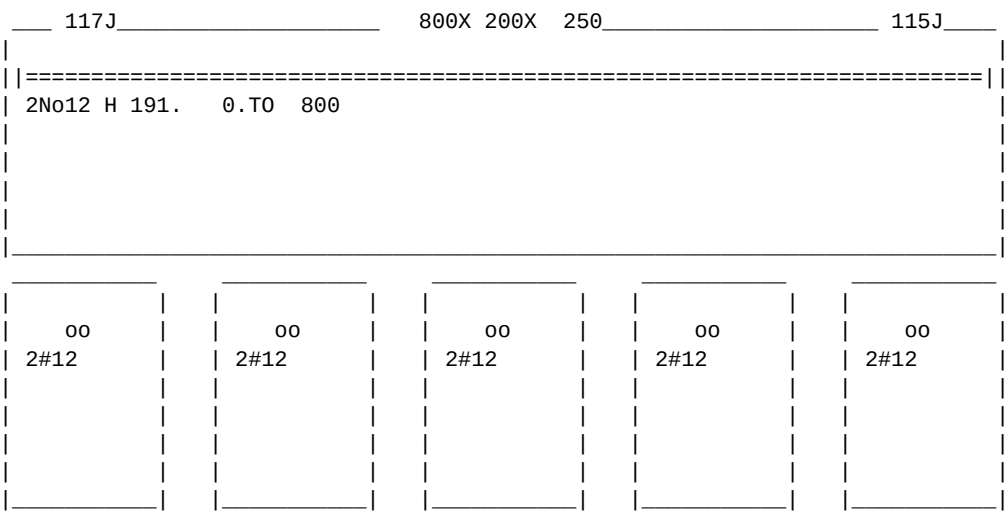
BEAM NO. 73 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 800. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	800.	YES	YES

B E A M N O. 73 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 3.97 KNS Tu= 3.1 KN-MET
Vc= 27.8 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 7 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 4.52 KNS Tu= 3.1 KN-MET
Vc= 27.8 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 7 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



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BEAM NO. 76 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

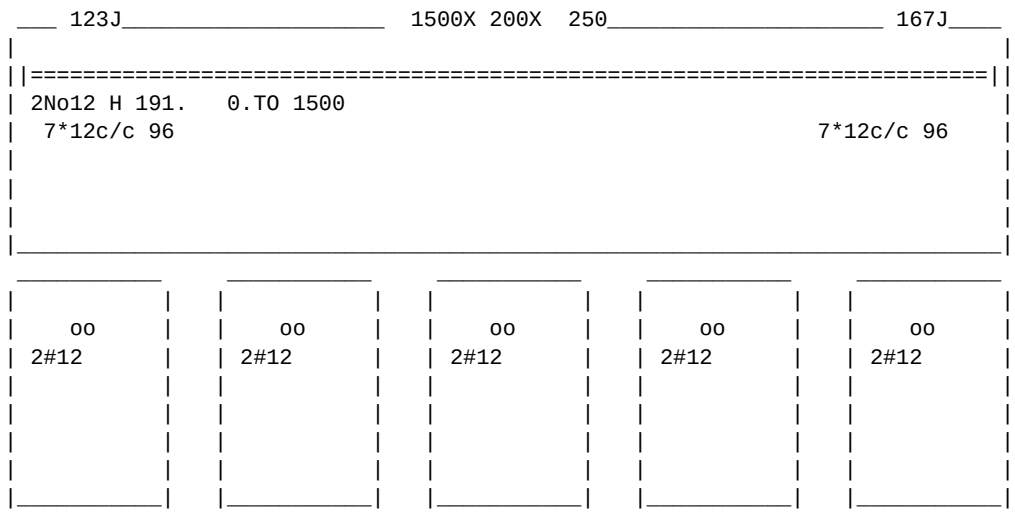
LEN - 1500. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	1500.	YES	YES

B E A M N O. 76 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 4.18 KNS Vc= 28.94 KNS Vs= 0.00 KNS
Tu= 0.12 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 565. MM

AT END SUPPORT - Vu= 5.68 KNS Vc= 28.85 KNS Vs= 0.00 KNS
Tu= 0.12 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 565. MM



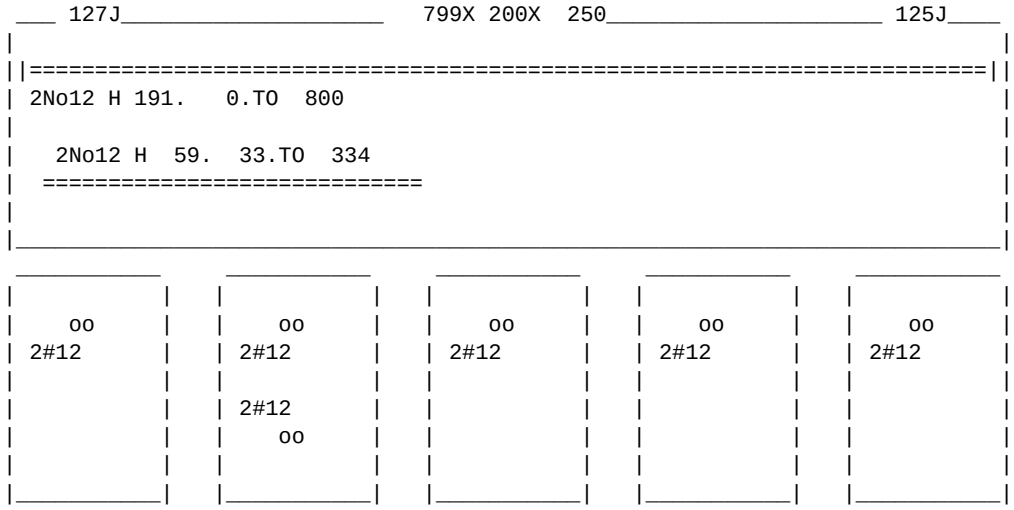
BEAM NO. 79 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 800. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	33.	334.	NO	NO
2	191.	2 - 12MM	0.	800.	YES	YES

B E A M N O. 79 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 3.33 KNS Tu= 2.7 KN-MET
Vc= 27.8 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 3.88 KNS Tu= 2.7 KN-MET
Vc= 27.8 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



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BEAM NO. 82 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

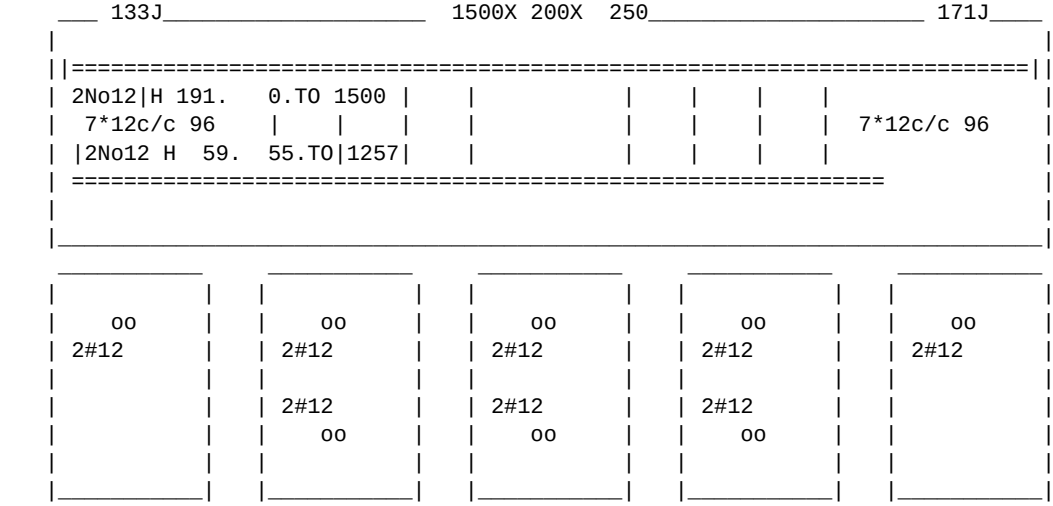
LEN - 1500. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	55.	1257.	NO	NO
2	191.	2 - 12MM	0.	1500.	YES	YES

B E A M N O. 82 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.09 KNS Vc= 29.08 KNS Vs= 0.00 KNS
Tu= 0.25 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 565. MM

AT END SUPPORT - Vu= 3.59 KNS Vc= 28.98 KNS Vs= 0.00 KNS
Tu= 0.25 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 565. MM



BEAM NO. 85 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

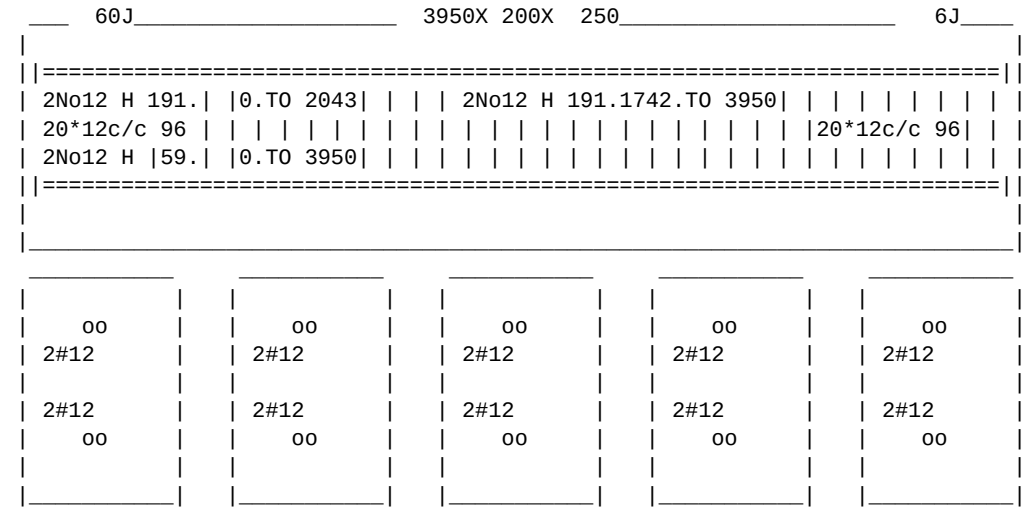
LEN - 3950. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3950.	YES	YES
2	191.	2 - 12MM	0.	2043.	YES	NO
3	191.	2 - 12MM	1742.	3950.	NO	YES

B E A M N O. 85 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 3.27 KNS Vc= 27.81 KNS Vs= 0.00 KNS
Tu= 0.20 KN-MET Tc= 0.7 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1790. MM

AT END SUPPORT - Vu= 1.80 KNS Vc= 27.81 KNS Vs= 0.00 KNS
Tu= 0.20 KN-MET Tc= 0.7 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1790. MM



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BEAM NO. 86 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

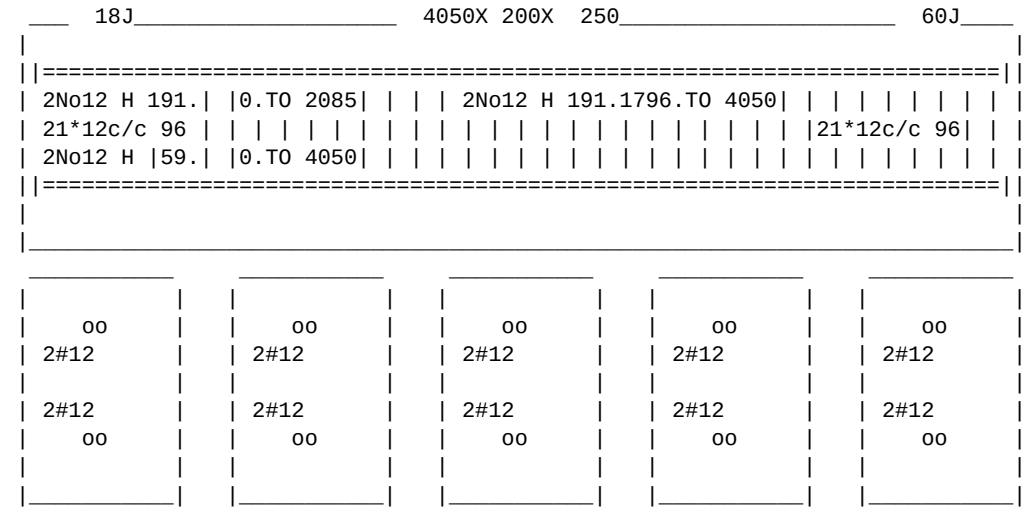
LEN - 4050. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	4050.	YES	YES
2	191.	2 - 12MM	0.	2085.	YES	NO
3	191.	2 - 12MM	1796.	4050.	NO	YES

B E A M N O. 86 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.57 KNS Vc= 28.57 KNS Vs= 0.00 KNS
Tu= 0.14 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1840. MM

AT END SUPPORT - Vu= 2.64 KNS Vc= 28.57 KNS Vs= 0.00 KNS
Tu= 0.14 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1840. MM



BEAM NO. 87 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

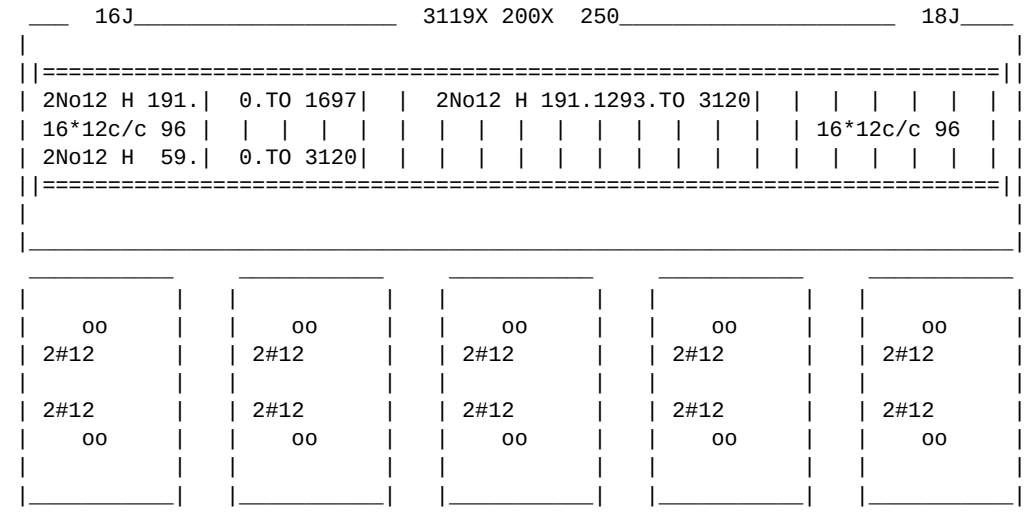
LEN - 3120. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3120.	YES	YES
2	191.	2 - 12MM	0.	1697.	YES	NO
3	191.	2 - 12MM	1293.	3120.	NO	YES

B E A M N O. 87 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 1.99 KNS Vc= 28.71 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1375. MM

AT END SUPPORT - Vu= 1.90 KNS Vc= 28.85 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1375. MM



BEAM NO. 88 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

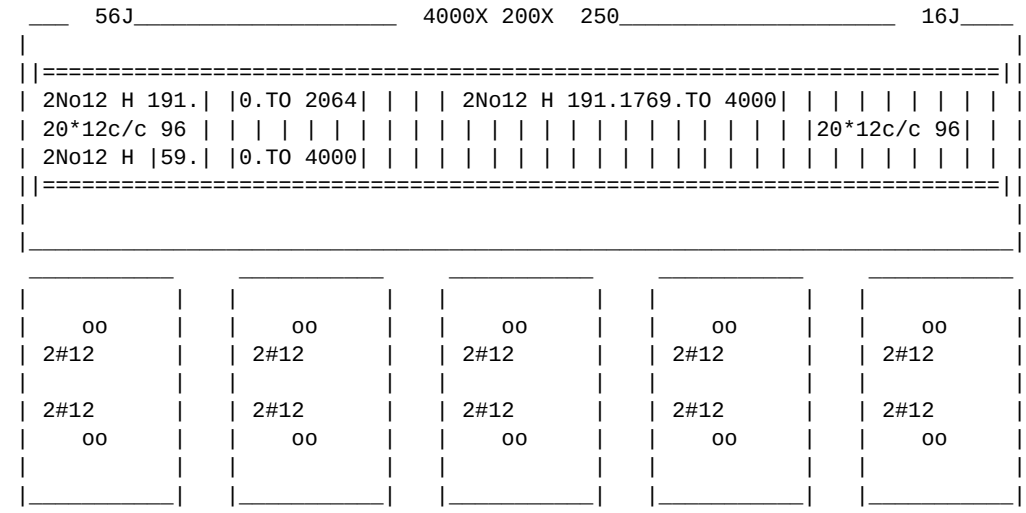
LEN - 4000. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	4000.	YES	YES
2	191.	2 - 12MM	0.	2064.	YES	NO
3	191.	2 - 12MM	1769.	4000.	NO	YES

B E A M N O. 88 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.74 KNS Vc= 28.96 KNS Vs= 0.00 KNS
Tu= 0.19 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1815. MM

AT END SUPPORT - Vu= 2.40 KNS Vc= 28.96 KNS Vs= 0.00 KNS
Tu= 0.19 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1815. MM



BEAM NO. 91 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

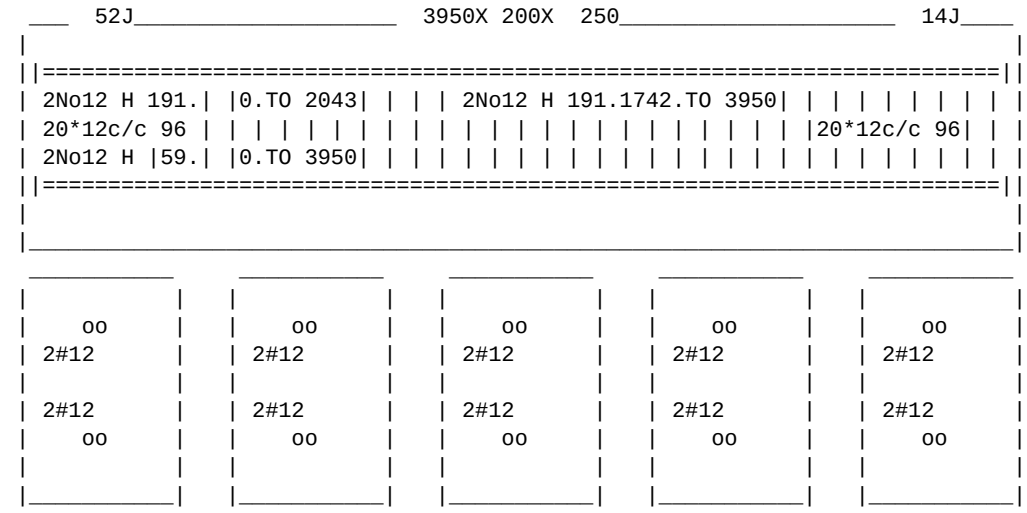
LEN - 3950. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3950.	YES	YES
2	191.	2 - 12MM	0.	2043.	YES	NO
3	191.	2 - 12MM	1742.	3950.	NO	YES

B E A M N O. 91 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.53 KNS Vc= 29.11 KNS Vs= 0.00 KNS
Tu= 0.19 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1790. MM

AT END SUPPORT - Vu= 2.54 KNS Vc= 29.11 KNS Vs= 0.00 KNS
Tu= 0.19 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1790. MM



BEAM NO. 92 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

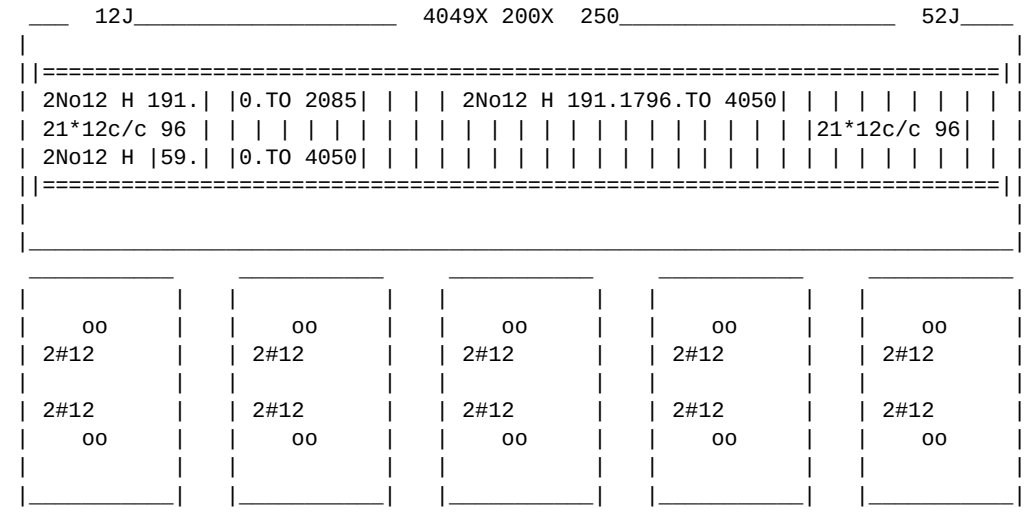
LEN - 4050. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	4050.	YES	YES
2	191.	2 - 12MM	0.	2085.	YES	NO
3	191.	2 - 12MM	1796.	4050.	NO	YES

B E A M N O. 92 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.42 KNS Vc= 28.95 KNS Vs= 0.00 KNS
Tu= 0.19 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1840. MM

AT END SUPPORT - Vu= 2.78 KNS Vc= 28.95 KNS Vs= 0.00 KNS
Tu= 0.19 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1840. MM



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BEAM NO. 93 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

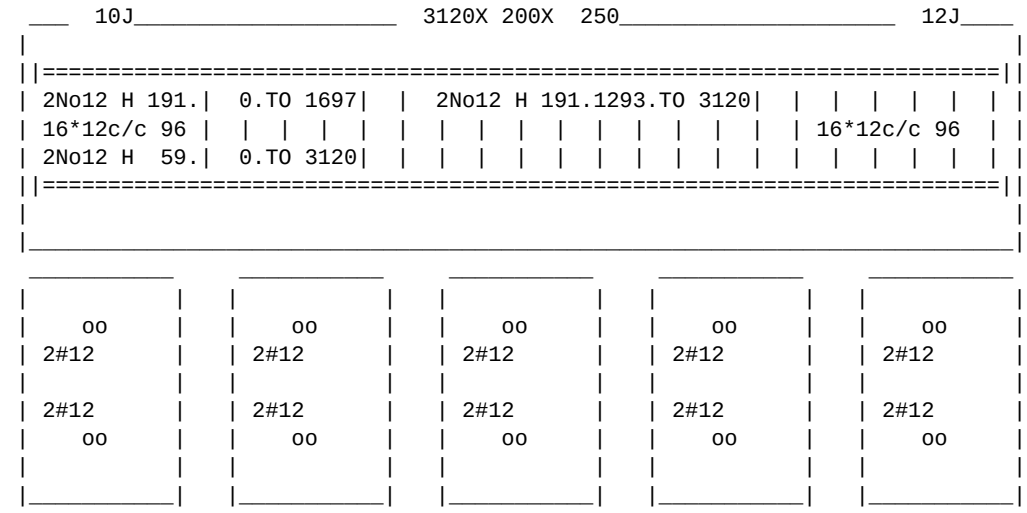
LEN - 3120. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3120.	YES	YES
2	191.	2 - 12MM	0.	1697.	YES	NO
3	191.	2 - 12MM	1293.	3120.	NO	YES

B E A M N O. 93 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 1.88 KNS Vc= 28.92 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1375. MM

AT END SUPPORT - Vu= 2.01 KNS Vc= 28.72 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1375. MM



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BEAM NO. 94 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

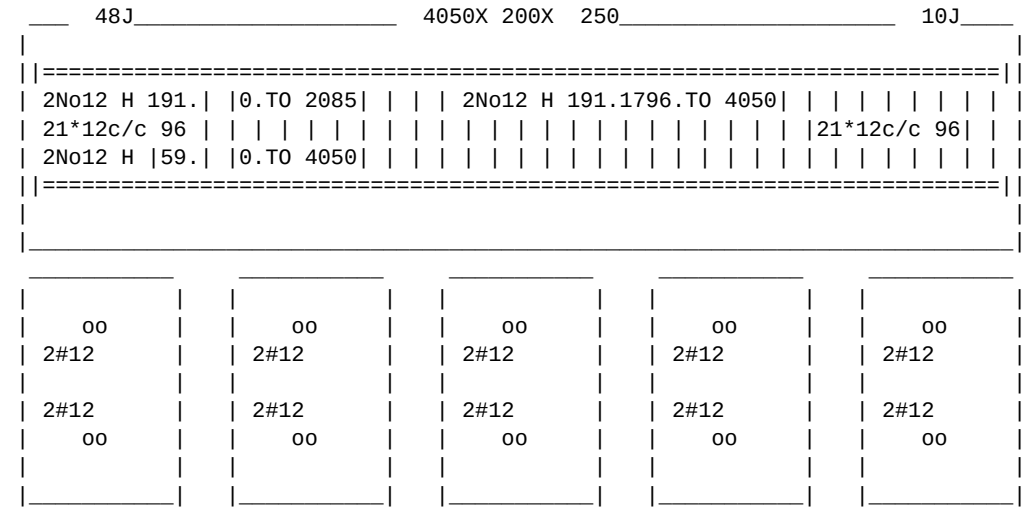
LEN - 4050. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	4050.	YES	YES
2	191.	2 - 12MM	0.	2085.	YES	NO
3	191.	2 - 12MM	1796.	4050.	NO	YES

B E A M N O. 94 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.64 KNS Vc= 28.59 KNS Vs= 0.00 KNS
Tu= 0.14 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1840. MM

AT END SUPPORT - Vu= 2.57 KNS Vc= 28.59 KNS Vs= 0.00 KNS
Tu= 0.14 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1840. MM



BEAM NO. 95 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

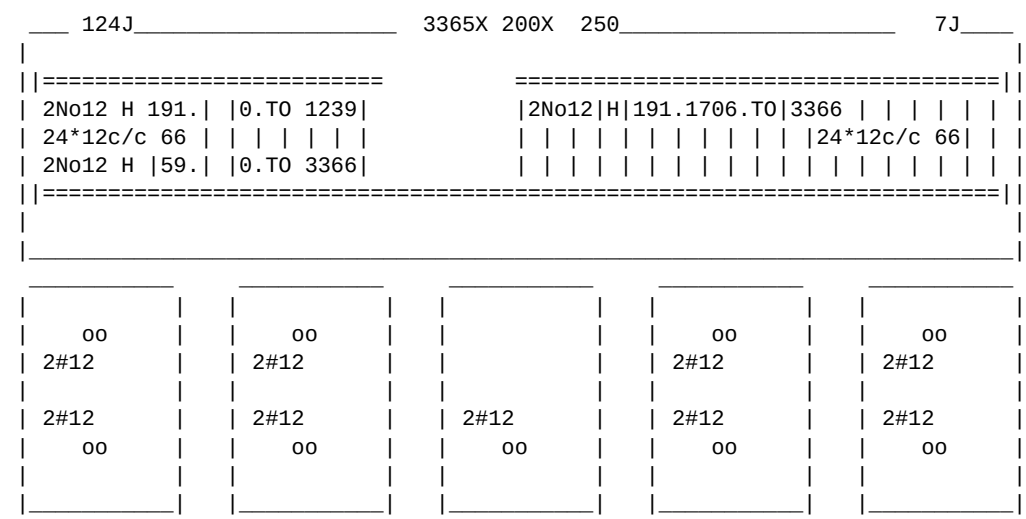
LEN - 3366. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3366.	YES	YES
2	191.	2 - 12MM	0.	1239.	YES	NO
3	191.	2 - 12MM	1706.	3366.	NO	YES

B E A M N O. 95 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 8.07 KNS Vc= 28.03 KNS Vs= 0.00 KNS
Tu= 1.08 KN-MET Tc= 0.8 KN-MET Ts= 1.4 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.63 SQ.CM.

AT END SUPPORT - Vu= 6.06 KNS Vc= 28.03 KNS Vs= 0.00 KNS
Tu= 1.08 KN-MET Tc= 0.8 KN-MET Ts= 1.4 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.63 SQ.CM.



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BEAM NO. 97 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

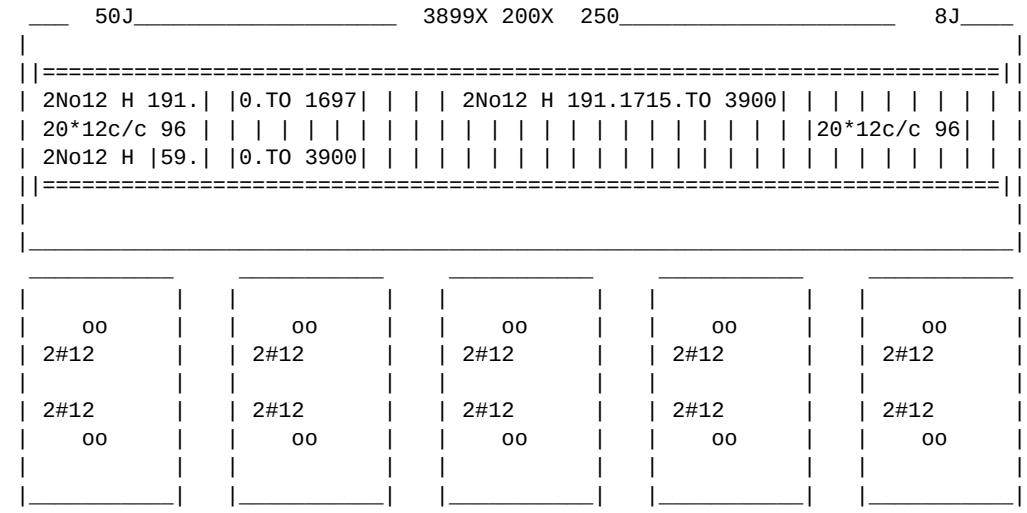
LEN - 3900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3900.	YES	YES
2	191.	2 - 12MM	0.	1697.	YES	NO
3	191.	2 - 12MM	1715.	3900.	NO	YES

B E A M N O. 97 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 3.00 KNS Vc= 28.86 KNS Vs= 0.00 KNS
Tu= 0.03 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1765. MM

AT END SUPPORT - Vu= 1.99 KNS Vc= 28.86 KNS Vs= 0.00 KNS
Tu= 0.03 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1765. MM



BEAM NO. 98 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

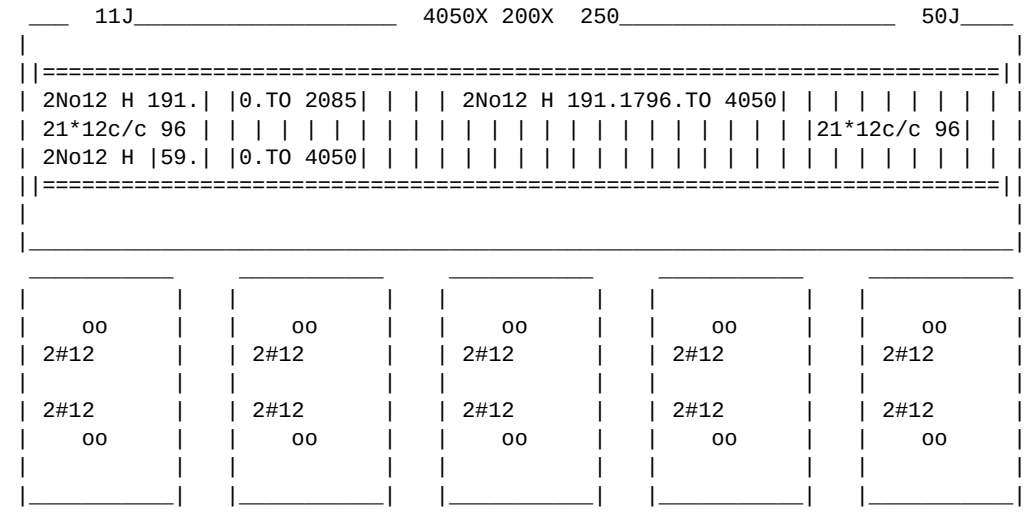
LEN - 4050. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	4050.	YES	YES
2	191.	2 - 12MM	0.	2085.	YES	NO
3	191.	2 - 12MM	1796.	4050.	NO	YES

B E A M N O. 98 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.44 KNS Vc= 28.98 KNS Vs= 0.00 KNS
Tu= 0.04 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1840. MM

AT END SUPPORT - Vu= 2.76 KNS Vc= 28.98 KNS Vs= 0.00 KNS
Tu= 0.04 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1840. MM



BEAM NO. 99 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

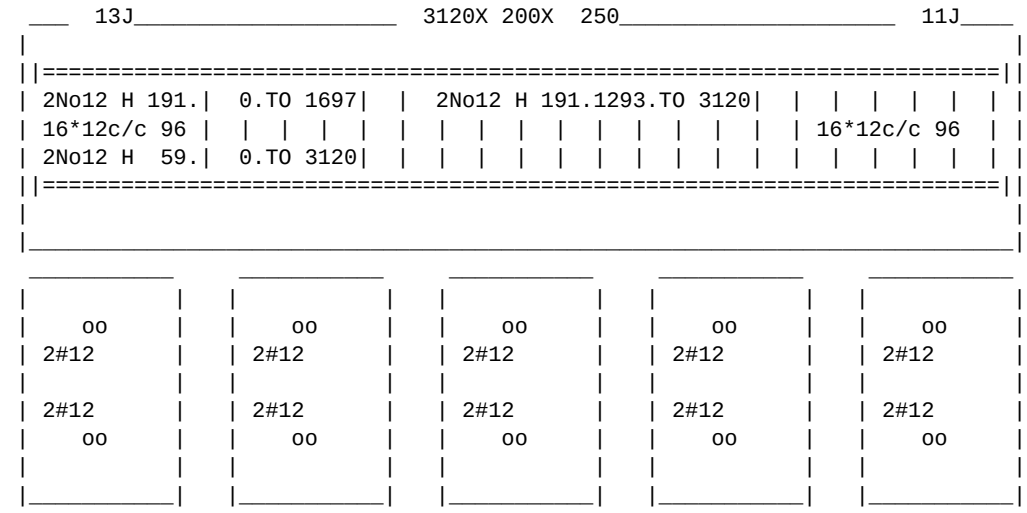
LEN - 3120. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3120.	YES	YES
2	191.	2 - 12MM	0.	1697.	YES	NO
3	191.	2 - 12MM	1293.	3120.	NO	YES

B E A M N O. 99 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 1.98 KNS Vc= 29.20 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1375. MM

AT END SUPPORT - Vu= 1.91 KNS Vc= 29.20 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1375. MM



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BEAM NO. 100 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

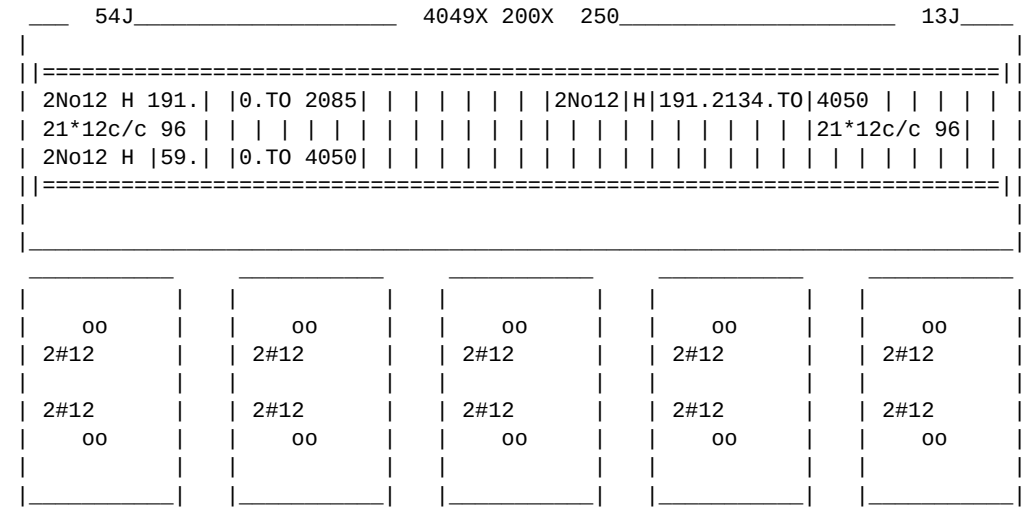
LEN - 4050. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	4050.	YES	YES
2	191.	2 - 12MM	0.	2085.	YES	NO
3	191.	2 - 12MM	2134.	4050.	NO	YES

B E A M N O. 100 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.75 KNS Vc= 29.06 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1840. MM

AT END SUPPORT - Vu= 2.46 KNS Vc= 29.06 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1840. MM



BEAM NO. 101 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

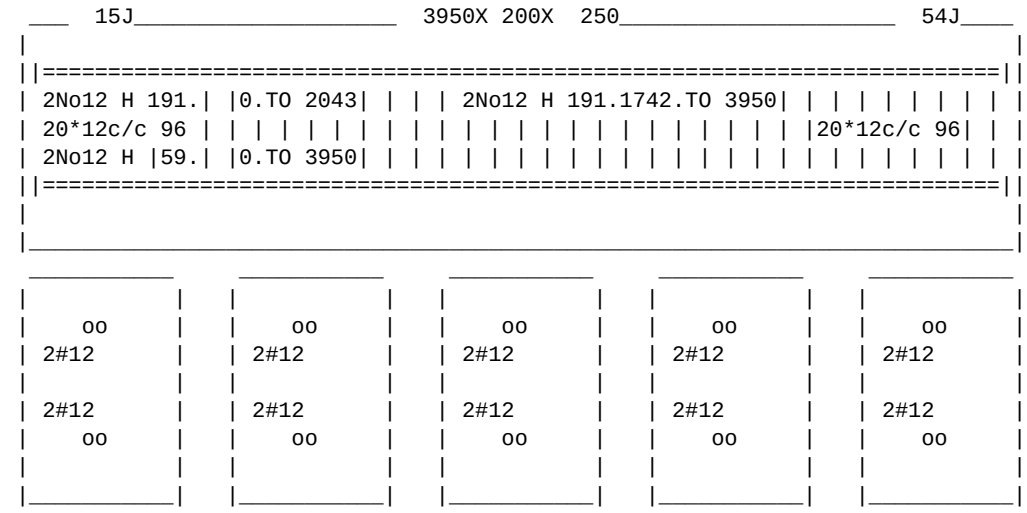
LEN - 3950. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3950.	YES	YES
2	191.	2 - 12MM	0.	2043.	YES	NO
3	191.	2 - 12MM	1742.	3950.	NO	YES

B E A M N O. 101 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.53 KNS Vc= 29.08 KNS Vs= 0.00 KNS
Tu= 0.04 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1790. MM

AT END SUPPORT - Vu= 2.53 KNS Vc= 29.08 KNS Vs= 0.00 KNS
Tu= 0.04 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1790. MM



BEAM NO. 104 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

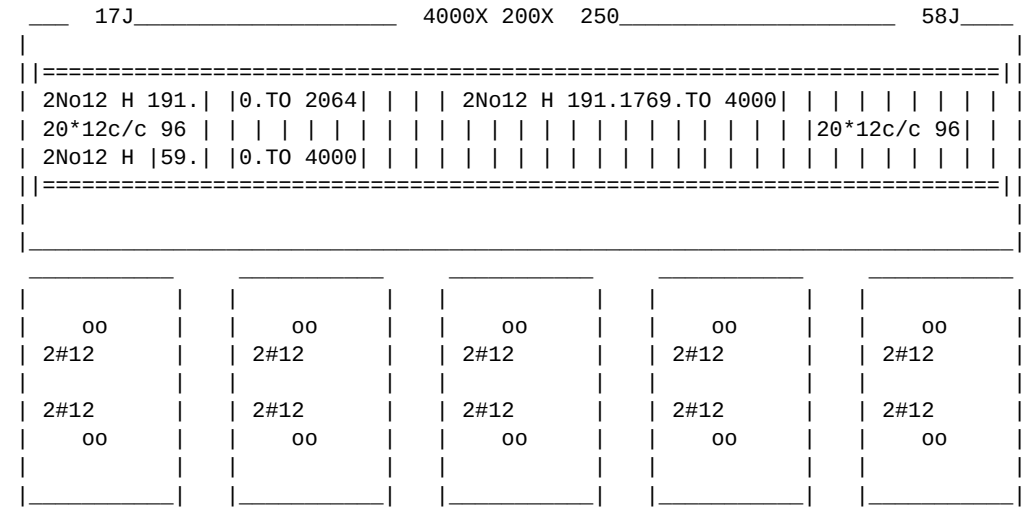
LEN - 4000. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	4000.	YES	YES
2	191.	2 - 12MM	0.	2064.	YES	NO
3	191.	2 - 12MM	1769.	4000.	NO	YES

B E A M N O. 104 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.42 KNS Vc= 29.06 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1815. MM

AT END SUPPORT - Vu= 2.71 KNS Vc= 29.06 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1815. MM



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BEAM NO. 105 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

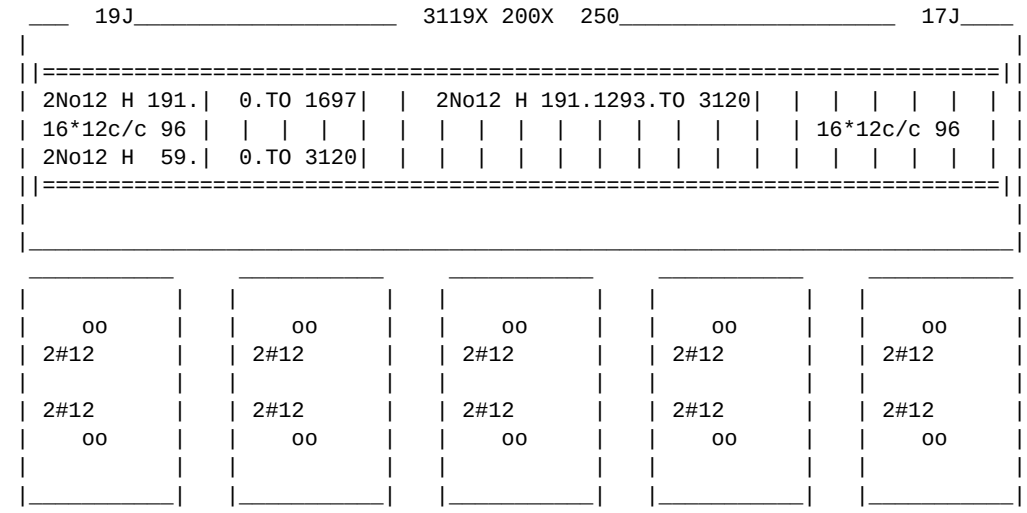
LEN - 3120. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3120.	YES	YES
2	191.	2 - 12MM	0.	1697.	YES	NO
3	191.	2 - 12MM	1293.	3120.	NO	YES

B E A M N O. 105 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 1.92 KNS Vc= 29.18 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1375. MM

AT END SUPPORT - Vu= 1.96 KNS Vc= 29.18 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1375. MM



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BEAM NO. 106 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

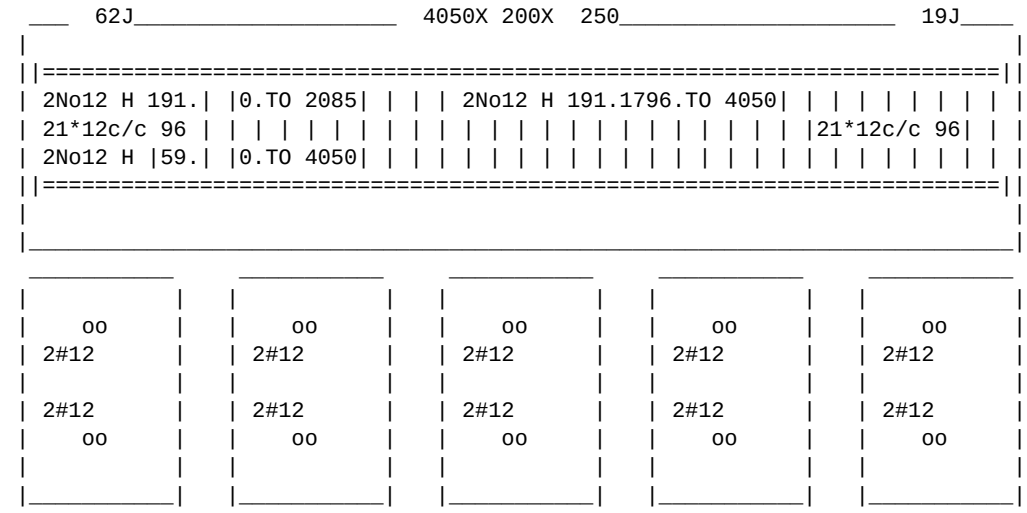
LEN - 4050. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	4050.	YES	YES
2	191.	2 - 12MM	0.	2085.	YES	NO
3	191.	2 - 12MM	1796.	4050.	NO	YES

B E A M N O. 106 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.76 KNS Vc= 28.97 KNS Vs= 0.00 KNS
Tu= 0.04 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1840. MM

AT END SUPPORT - Vu= 2.44 KNS Vc= 28.97 KNS Vs= 0.00 KNS
Tu= 0.04 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1840. MM



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BEAM NO. 107 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

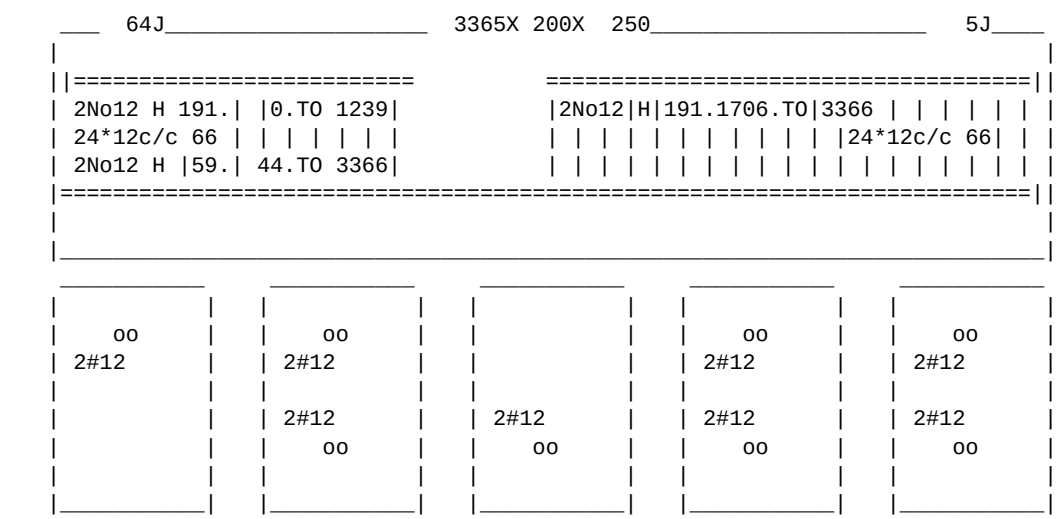
LEN - 3366. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	44.	3366.	NO	YES
2	191.	2 - 12MM	0.	1239.	YES	NO
3	191.	2 - 12MM	1706.	3366.	NO	YES

B E A M N O. 107 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 8.51 KNS Vc= 27.77 KNS Vs= 0.00 KNS
Tu= 1.00 KN-MET Tc= 0.7 KN-MET Ts= 1.3 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.58 SQ.CM.

AT END SUPPORT - Vu= 5.61 KNS Vc= 27.77 KNS Vs= 0.00 KNS
Tu= 1.00 KN-MET Tc= 0.7 KN-MET Ts= 1.3 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.58 SQ.CM.



BEAM NO. 108 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

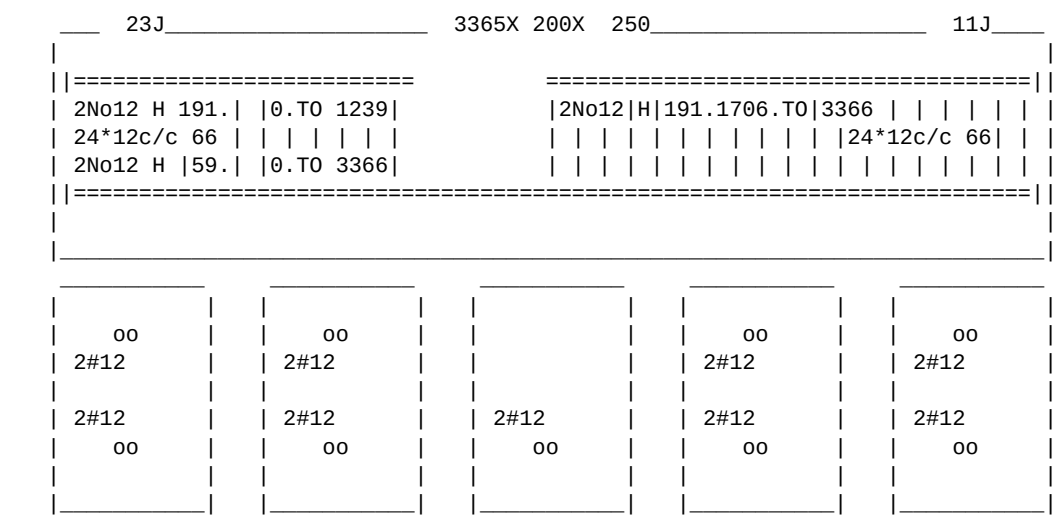
LEN - 3366. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3366.	YES	YES
2	191.	2 - 12MM	0.	1239.	YES	NO
3	191.	2 - 12MM	1706.	3366.	NO	YES

B E A M N O. 108 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 8.13 KNS Vc= 27.87 KNS Vs= 0.00 KNS
Tu= 1.17 KN-MET Tc= 0.7 KN-MET Ts= 1.6 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.68 SQ.CM.

AT END SUPPORT - Vu= 6.00 KNS Vc= 27.87 KNS Vs= 0.00 KNS
Tu= 1.17 KN-MET Tc= 0.7 KN-MET Ts= 1.6 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.68 SQ.CM.



BEAM NO. 109 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

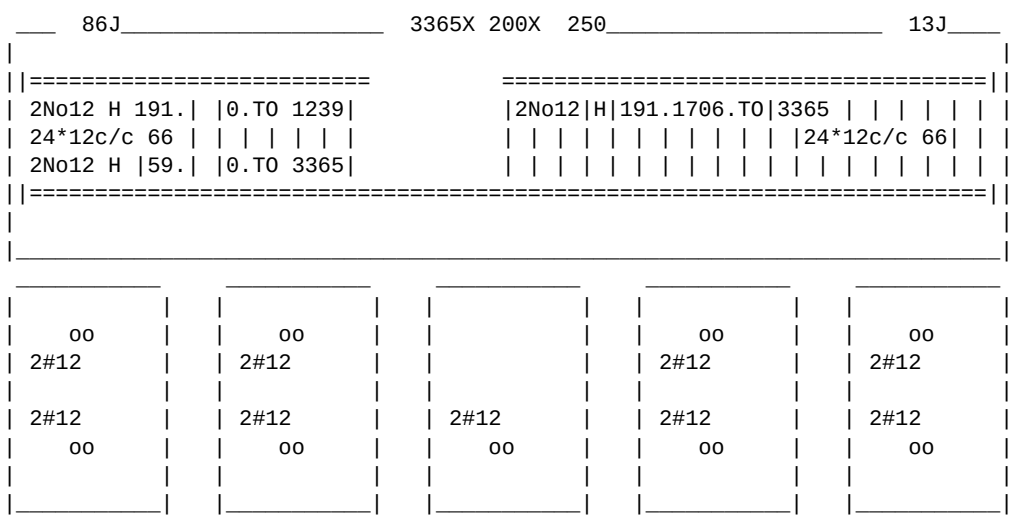
LEN - 3365. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3365.	YES	YES
2	191.	2 - 12MM	0.	1239.	YES	NO
3	191.	2 - 12MM	1706.	3365.	NO	YES

B E A M N O. 109 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 10.04 KNS Vc= 27.98 KNS Vs= 0.00 KNS
Tu= 1.17 KN-MET Tc= 0.8 KN-MET Ts= 1.6 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.68 SQ.CM.

AT END SUPPORT - Vu= 4.08 KNS Vc= 27.98 KNS Vs= 0.00 KNS
Tu= 1.17 KN-MET Tc= 0.8 KN-MET Ts= 1.6 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.68 SQ.CM.



BEAM NO. 112 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

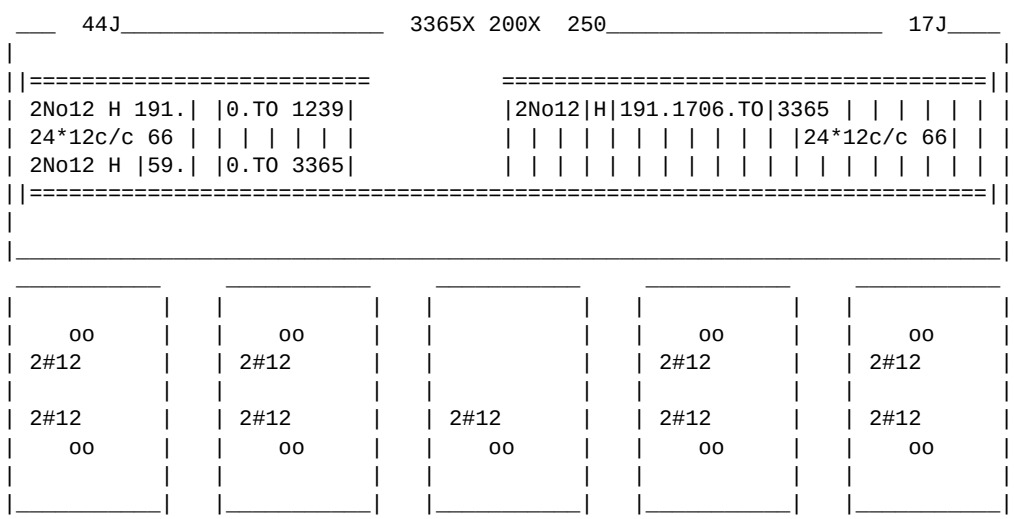
LEN - 3365. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3365.	YES	YES
2	191.	2 - 12MM	0.	1239.	YES	NO
3	191.	2 - 12MM	1706.	3365.	NO	YES

B E A M N O. 112 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 8.14 KNS Vc= 27.90 KNS Vs= 0.00 KNS
Tu= 1.16 KN-MET Tc= 0.8 KN-MET Ts= 1.6 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.68 SQ.CM.

AT END SUPPORT - Vu= 5.98 KNS Vc= 27.90 KNS Vs= 0.00 KNS
Tu= 1.16 KN-MET Tc= 0.8 KN-MET Ts= 1.6 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.68 SQ.CM.



BEAM NO. 113 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

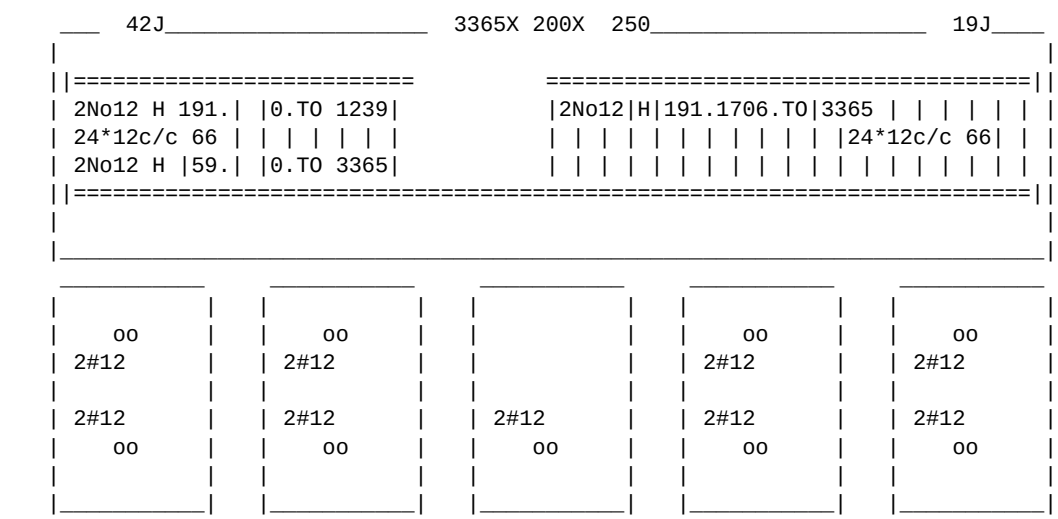
LEN - 3365. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3365.	YES	YES
2	191.	2 - 12MM	0.	1239.	YES	NO
3	191.	2 - 12MM	1706.	3365.	NO	YES

B E A M N O. 113 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 10.00 KNS Vc= 27.94 KNS Vs= 0.00 KNS
Tu= 1.17 KN-MET Tc= 0.8 KN-MET Ts= 1.6 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.68 SQ.CM.

AT END SUPPORT - Vu= 4.12 KNS Vc= 27.94 KNS Vs= 0.00 KNS
Tu= 1.17 KN-MET Tc= 0.8 KN-MET Ts= 1.6 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.68 SQ.CM.



BEAM NO. 115 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

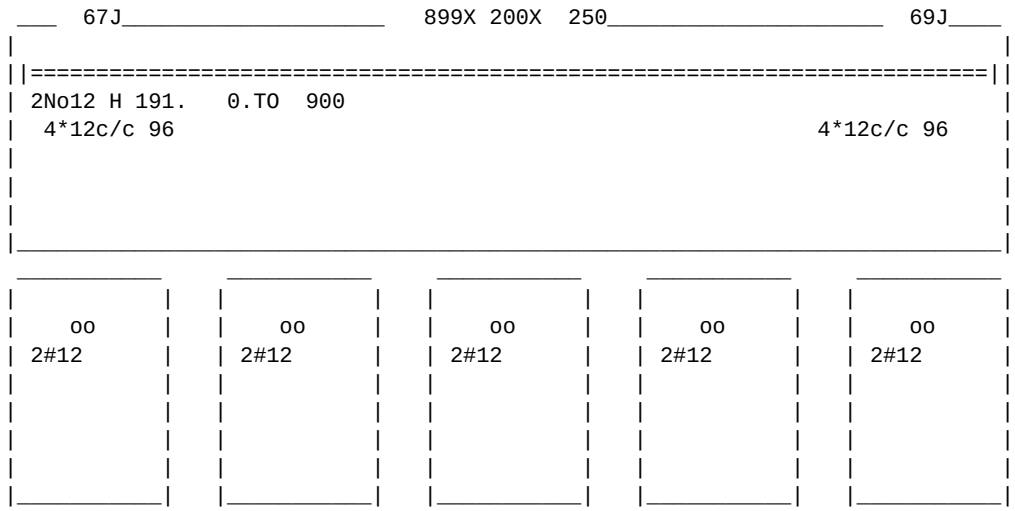
LEN - 900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	900.	YES	YES

B E A M N O. 115 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 4.76 KNS Vc= 28.83 KNS Vs= 0.00 KNS
Tu= 0.04 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 265. MM

AT END SUPPORT - Vu= 4.09 KNS Vc= 28.88 KNS Vs= 0.00 KNS
Tu= 0.04 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 265. MM



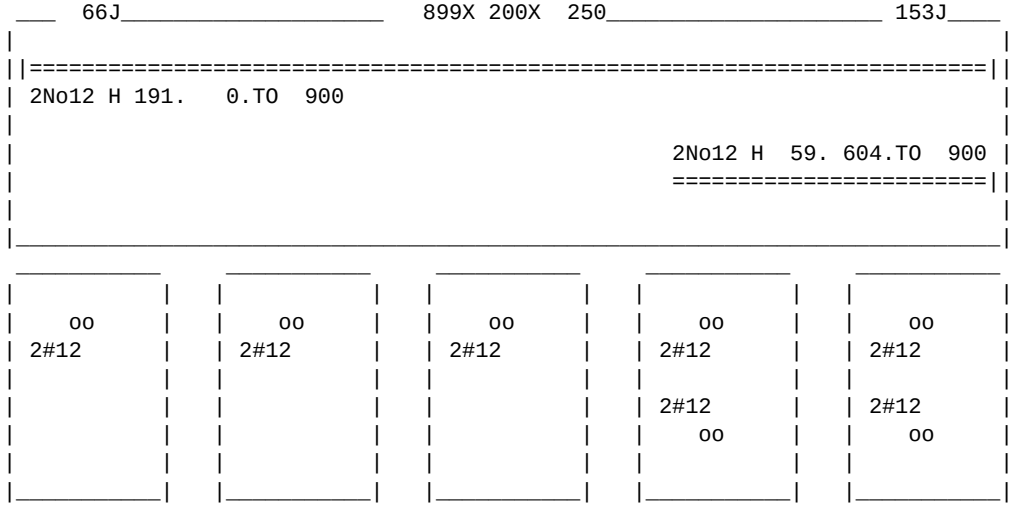
BEAM NO. 116 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	604.	900.	NO	YES
2	191.	2 - 12MM	0.	900.	YES	YES

B E A M N O. 116 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 14.86 KNS Tu= 3.8 KN-MET
Vc= 28.2 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 6 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 14.19 KNS Tu= 3.8 KN-MET
Vc= 51.4 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 6 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



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BEAM NO. 117 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

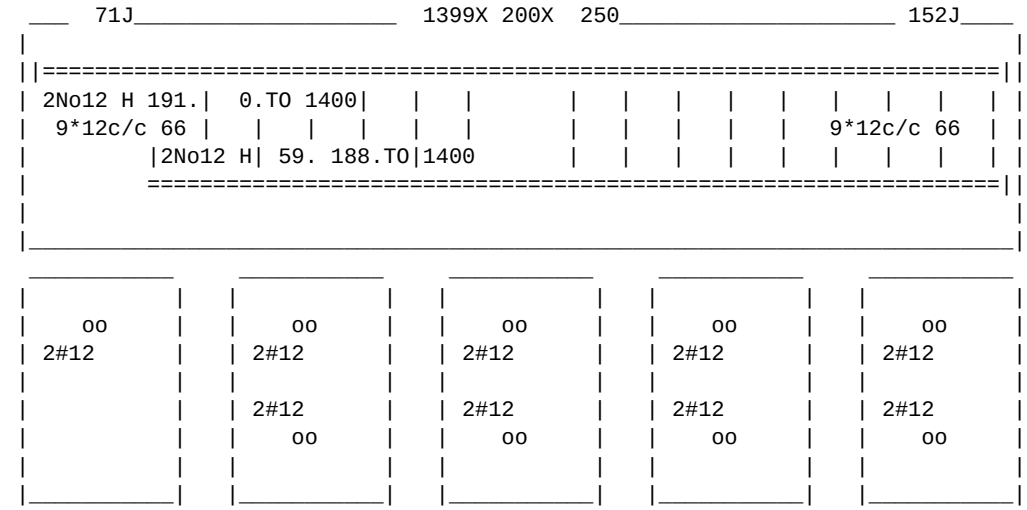
LEN - 1400. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	188.	1400.	NO	YES
2	191.	2 - 12MM	0.	1400.	YES	YES

B E A M N O. 117 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 11.51 KNS Vc= 28.59 KNS Vs= 0.00 KNS
Tu= 2.24 KN-MET Tc= 0.9 KN-MET Ts= 3.0 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 515. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.30 SQ.CM.

AT END SUPPORT - Vu= 10.15 KNS Vc= 29.27 KNS Vs= 0.00 KNS
Tu= 2.24 KN-MET Tc= 0.9 KN-MET Ts= 3.0 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 515. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.30 SQ.CM.



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BEAM NO. 119 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

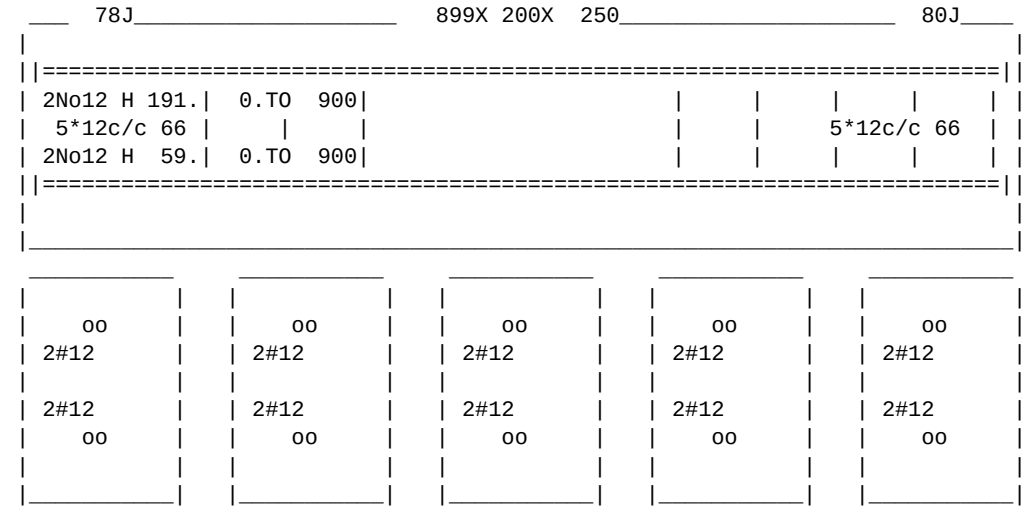
LEN - 900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	900.	YES	YES
2	191.	2 - 12MM	0.	900.	YES	YES

B E A M N O. 119 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.69 KNS Vc= 28.97 KNS Vs= 0.00 KNS
Tu= 1.00 KN-MET Tc= 0.8 KN-MET Ts= 1.3 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 265. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.58 SQ.CM.

AT END SUPPORT - Vu= 2.02 KNS Vc= 29.02 KNS Vs= 0.00 KNS
Tu= 1.00 KN-MET Tc= 0.8 KN-MET Ts= 1.3 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 265. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.58 SQ.CM.



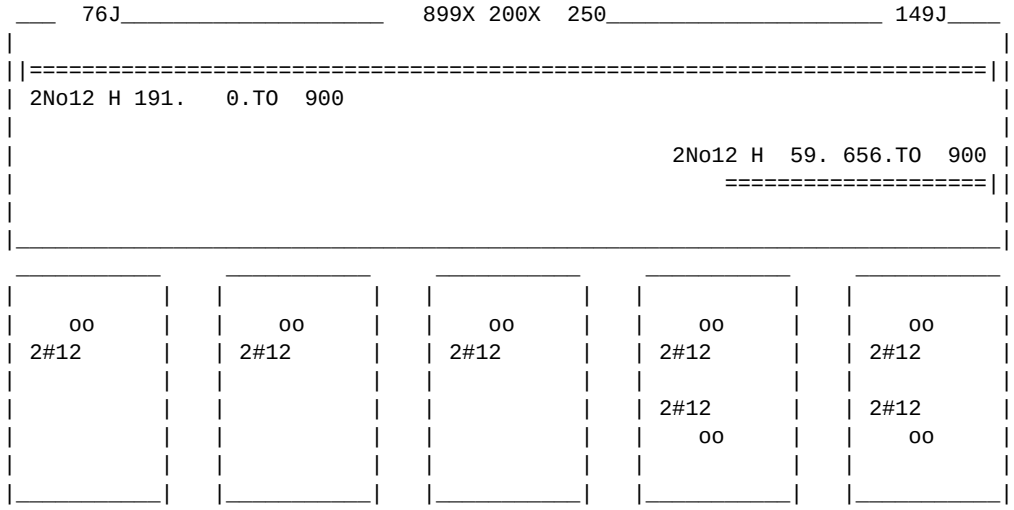
BEAM NO. 120 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	656.	900.	NO	YES
2	191.	2 - 12MM	0.	900.	YES	YES

B E A M N O. 120 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 11.31 KNS Tu= 4.3 KN-MET
Vc= 28.0 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 10.63 KNS Tu= 4.3 KN-MET
Vc= 48.3 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



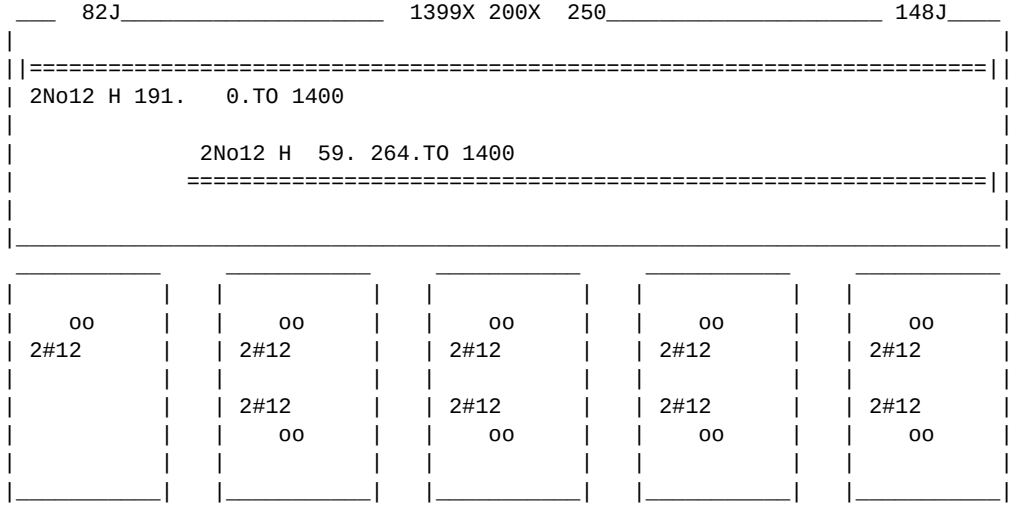
BEAM NO. 121 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 1400. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	264.	1400.	NO	YES
2	191.	2 - 12MM	0.	1400.	YES	YES

B E A M N O. 121 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 8.58 KNS Tu= 3.2 KN-MET
Vc= 28.3 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 7.21 KNS Tu= 3.2 KN-MET
Vc= 29.4 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



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BEAM NO. 123 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

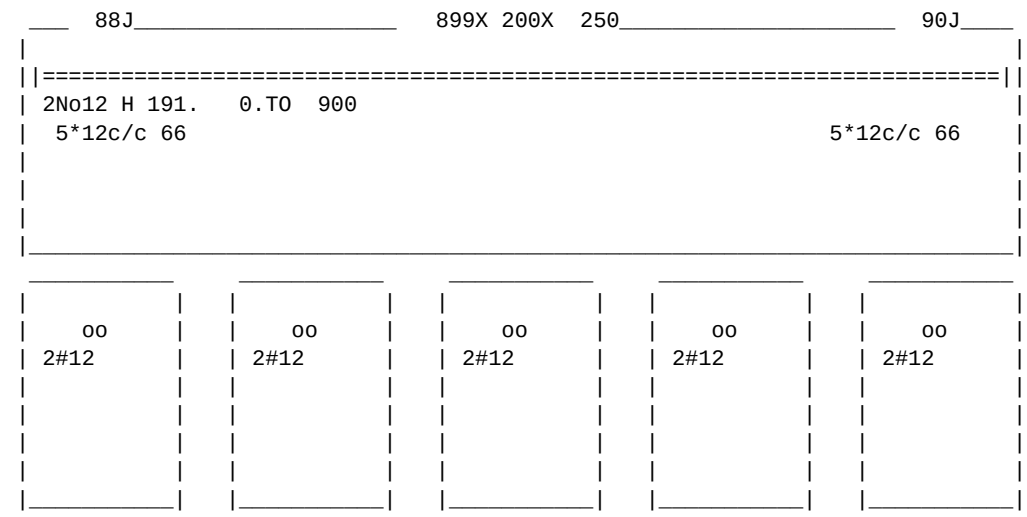
LEN - 900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	900.	YES	YES

B E A M N O. 123 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 3.93 KNS Vc= 28.87 KNS Vs= 0.00 KNS
Tu= 0.82 KN-MET Tc= 0.8 KN-MET Ts= 1.1 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 265. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.48 SQ.CM.

AT END SUPPORT - Vu= 3.25 KNS Vc= 28.92 KNS Vs= 0.00 KNS
Tu= 0.82 KN-MET Tc= 0.8 KN-MET Ts= 1.1 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 265. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.48 SQ.CM.



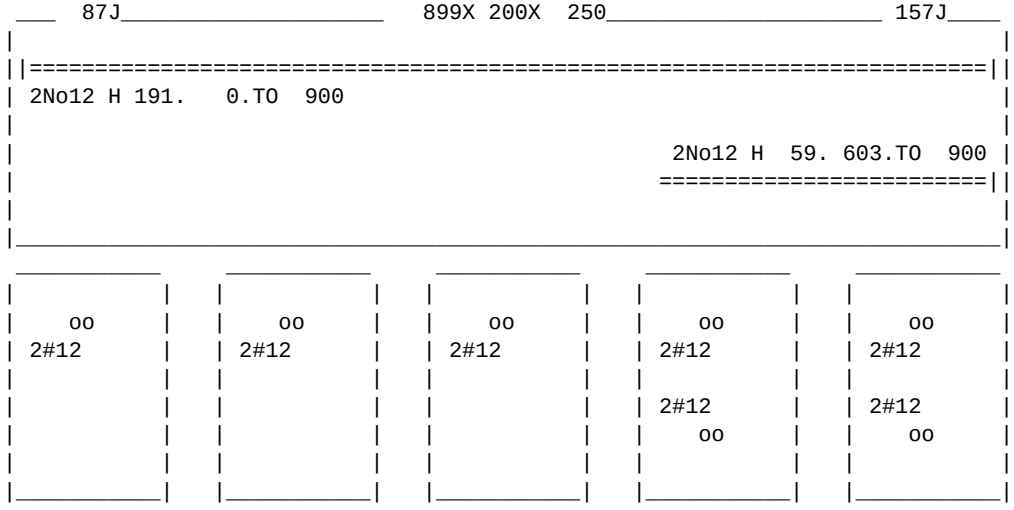
BEAM NO. 124 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	603.	900.	NO	YES
2	191.	2 - 12MM	0.	900.	YES	YES

B E A M N O. 124 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 15.06 KNS Tu= 3.0 KN-MET
Vc= 28.2 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 7 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 15.63 KNS Tu= 3.8 KN-MET
Vc= 51.4 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 9 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



BEAM NO. 125 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

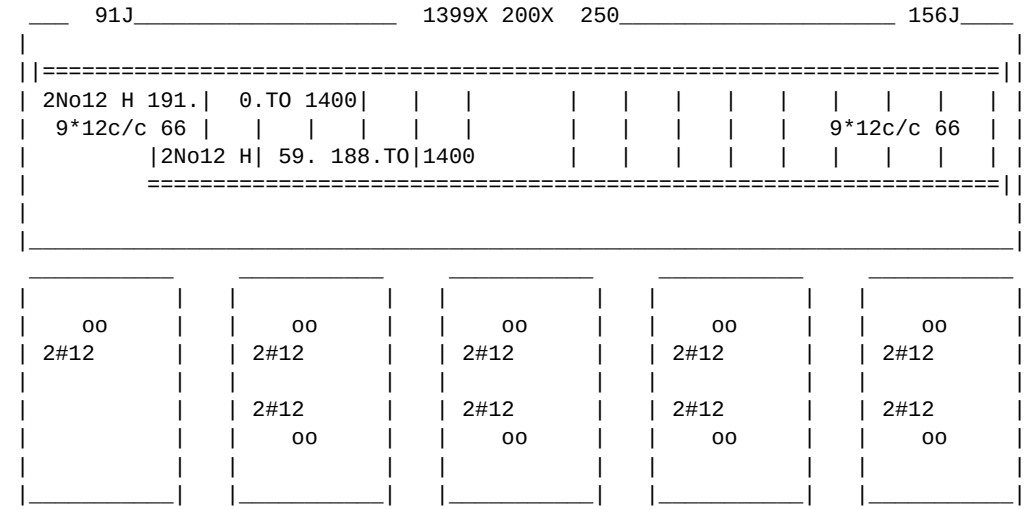
LEN - 1400. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	188.	1400.	NO	YES
2	191.	2 - 12MM	0.	1400.	YES	YES

B E A M N O. 125 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 11.61 KNS Vc= 28.59 KNS Vs= 0.00 KNS
Tu= 2.14 KN-MET Tc= 0.9 KN-MET Ts= 2.8 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 515. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.24 SQ.CM.

AT END SUPPORT - Vu= 10.24 KNS Vc= 29.28 KNS Vs= 0.00 KNS
Tu= 2.14 KN-MET Tc= 0.9 KN-MET Ts= 2.8 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 515. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.24 SQ.CM.



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BEAM NO. 131 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

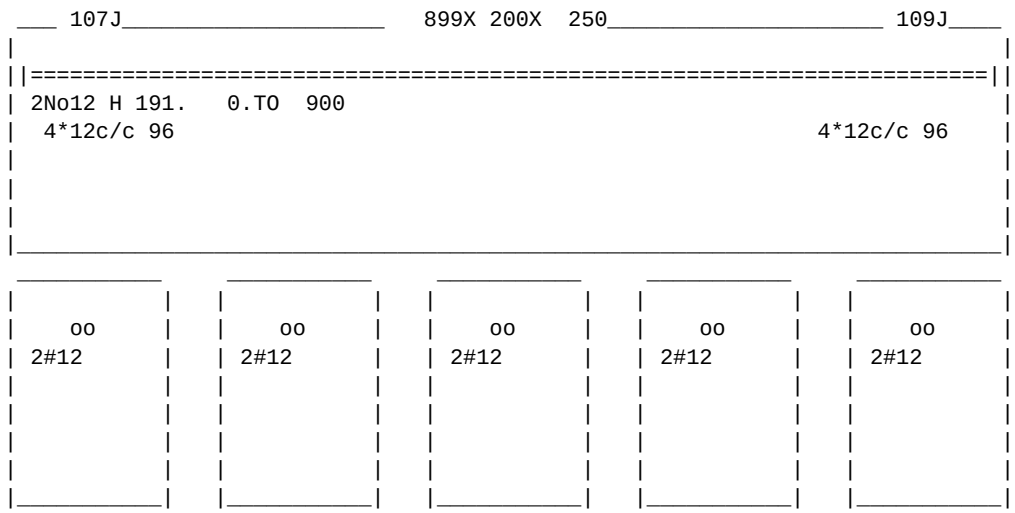
LEN - 900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	900.	YES	YES

B E A M N O. 131 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 4.81 KNS Vc= 28.82 KNS Vs= 0.00 KNS
Tu= 0.04 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 265. MM

AT END SUPPORT - Vu= 4.13 KNS Vc= 28.88 KNS Vs= 0.00 KNS
Tu= 0.04 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 265. MM



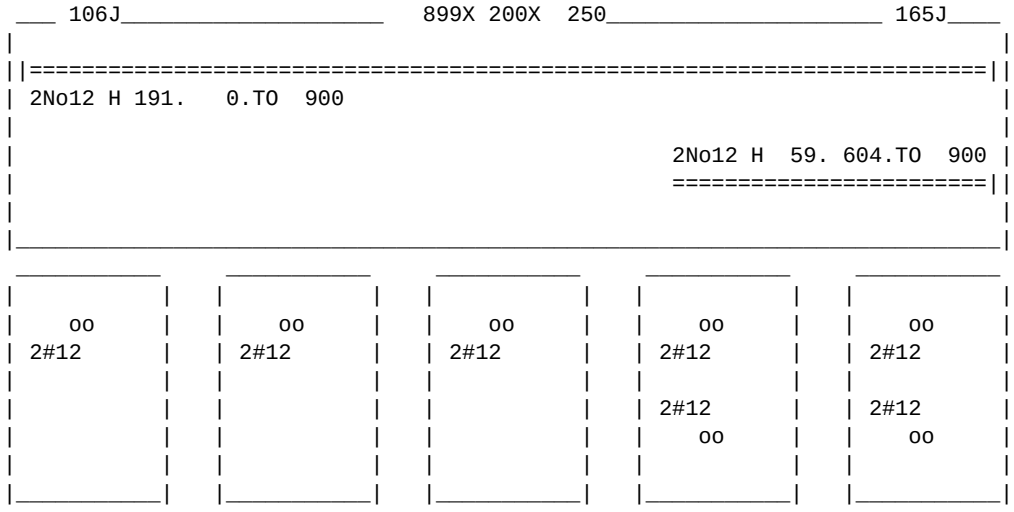
BEAM NO. 132 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	604.	900.	NO	YES
2	191.	2 - 12MM	0.	900.	YES	YES

B E A M N O. 132 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 14.84 KNS Tu= 3.6 KN-MET
Vc= 28.2 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 6 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 14.16 KNS Tu= 3.6 KN-MET
Vc= 51.4 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 6 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



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BEAM NO. 133 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

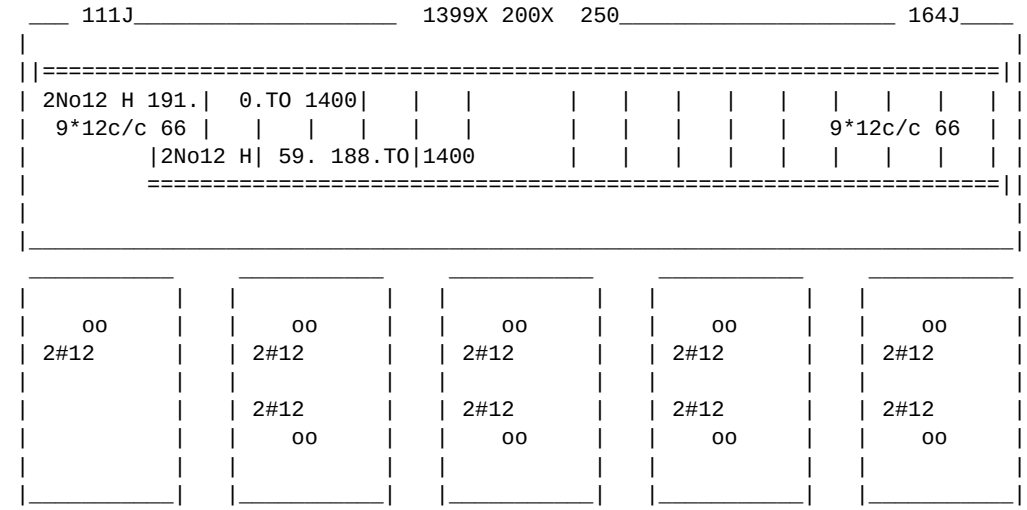
LEN - 1400. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	188.	1400.	NO	YES
2	191.	2 - 12MM	0.	1400.	YES	YES

B E A M N O. 133 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 11.49 KNS Vc= 28.59 KNS Vs= 0.00 KNS
Tu= 2.12 KN-MET Tc= 0.9 KN-MET Ts= 2.8 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 515. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.23 SQ.CM.

AT END SUPPORT - Vu= 10.12 KNS Vc= 29.28 KNS Vs= 0.00 KNS
Tu= 2.12 KN-MET Tc= 0.9 KN-MET Ts= 2.8 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 515. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.23 SQ.CM.



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BEAM NO. 135 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

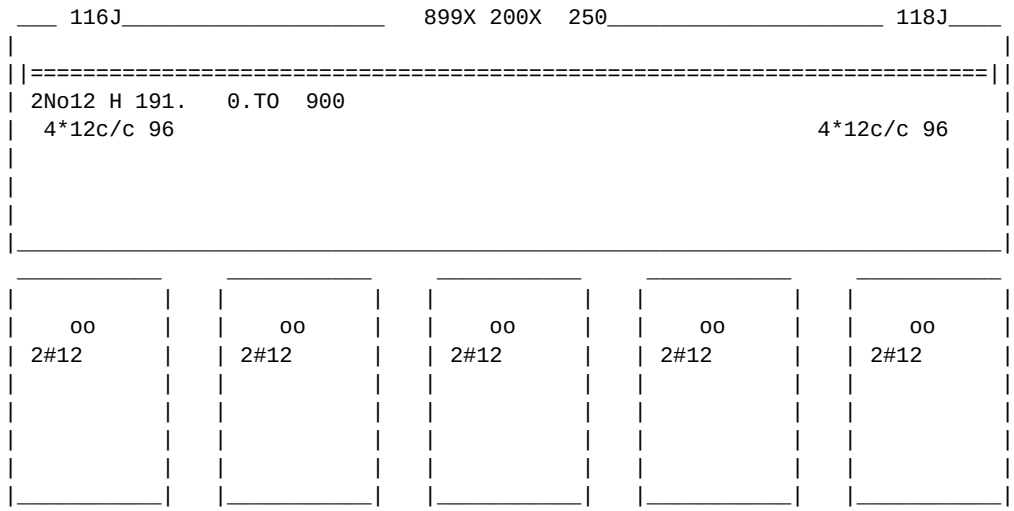
LEN - 900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	900.	YES	YES

B E A M N O. 135 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 4.77 KNS Vc= 28.83 KNS Vs= 0.00 KNS
Tu= 0.03 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 265. MM

AT END SUPPORT - Vu= 4.09 KNS Vc= 28.88 KNS Vs= 0.00 KNS
Tu= 0.03 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 265. MM



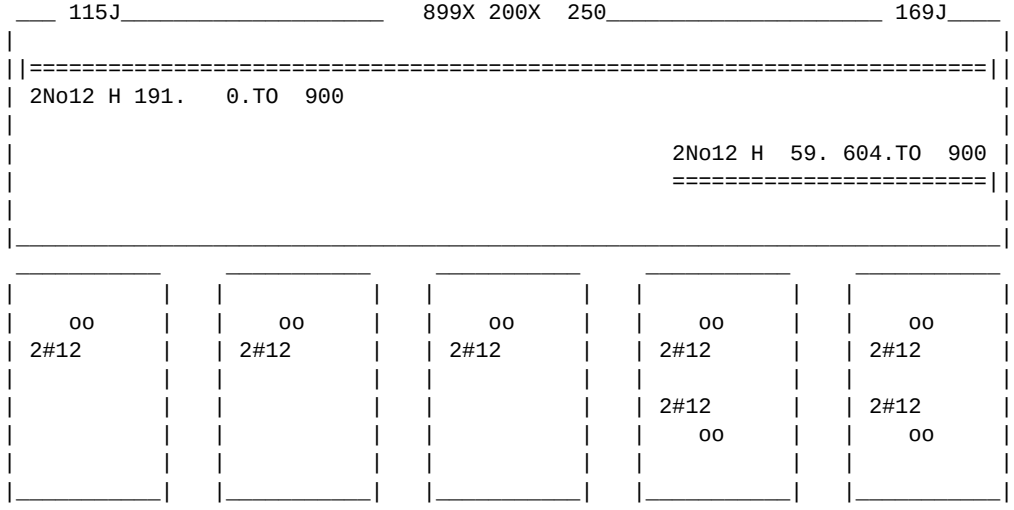
BEAM NO. 136 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	604.	900.	NO	YES
2	191.	2 - 12MM	0.	900.	YES	YES

B E A M N O. 136 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 14.94 KNS Tu= 3.2 KN-MET
Vc= 28.2 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 7 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 14.27 KNS Tu= 3.2 KN-MET
Vc= 51.4 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 7 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



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BEAM NO. 137 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

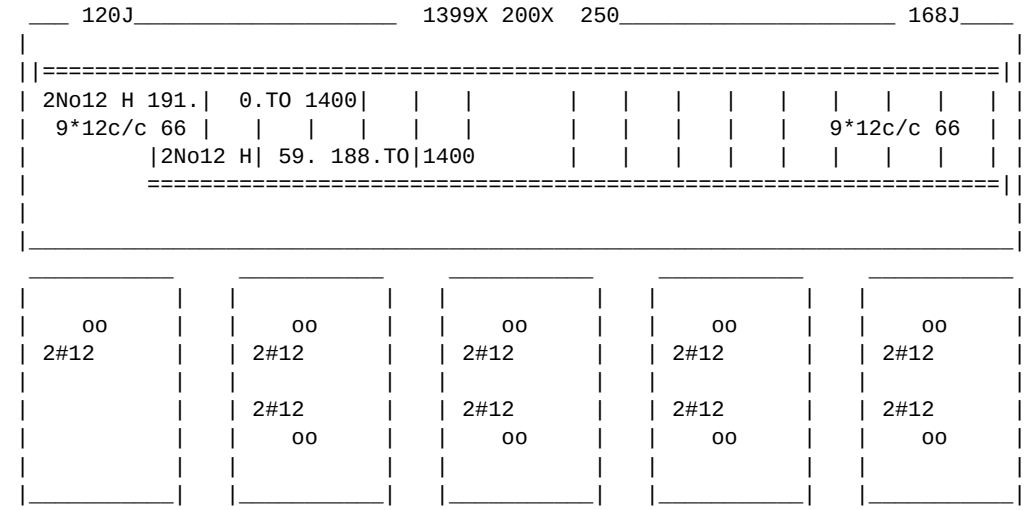
LEN - 1400. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	188.	1400.	NO	YES
2	191.	2 - 12MM	0.	1400.	YES	YES

B E A M N O. 137 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 11.54 KNS Vc= 28.60 KNS Vs= 0.00 KNS
Tu= 2.20 KN-MET Tc= 0.9 KN-MET Ts= 2.9 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 515. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.28 SQ.CM.

AT END SUPPORT - Vu= 10.17 KNS Vc= 29.26 KNS Vs= 0.00 KNS
Tu= 2.20 KN-MET Tc= 0.9 KN-MET Ts= 2.9 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 515. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.28 SQ.CM.



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BEAM NO. 139 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

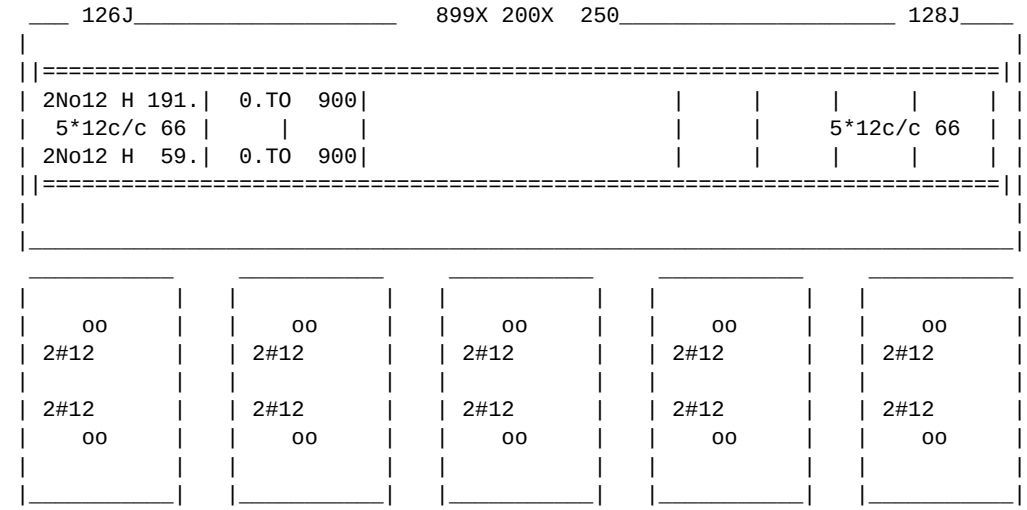
LEN - 900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	900.	YES	YES
2	191.	2 - 12MM	0.	900.	YES	YES

B E A M N O. 139 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.63 KNS Vc= 28.97 KNS Vs= 0.00 KNS
Tu= 0.96 KN-MET Tc= 0.8 KN-MET Ts= 1.3 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 265. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.56 SQ.CM.

AT END SUPPORT - Vu= 1.95 KNS Vc= 29.02 KNS Vs= 0.00 KNS
Tu= 0.96 KN-MET Tc= 0.8 KN-MET Ts= 1.3 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 265. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.56 SQ.CM.



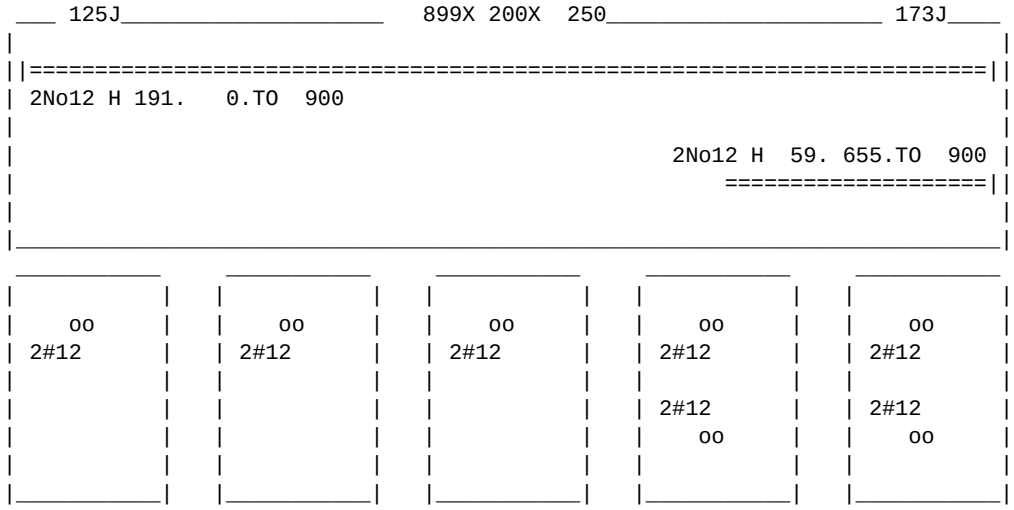
BEAM NO. 140 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	655.	900.	NO	YES
2	191.	2 - 12MM	0.	900.	YES	YES

B E A M N O. 140 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 11.43 KNS Tu= 4.4 KN-MET
Vc= 28.0 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 10.75 KNS Tu= 4.4 KN-MET
Vc= 51.4 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



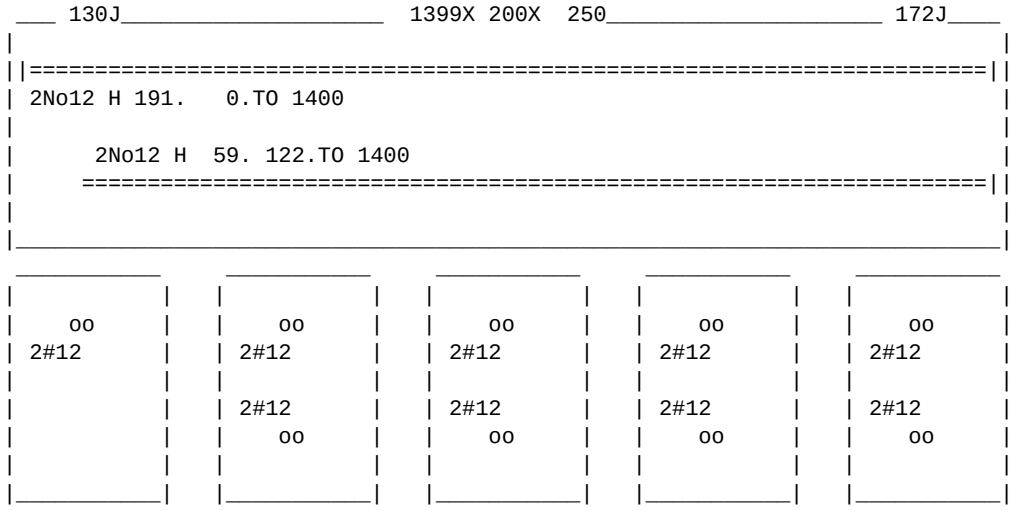
BEAM NO. 141 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 1400. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	122.	1400.	NO	YES
2	191.	2 - 12MM	0.	1400.	YES	YES

B E A M N O. 141 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 8.66 KNS Tu= 3.3 KN-MET
Vc= 28.4 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 7.30 KNS Tu= 3.3 KN-MET
Vc= 29.7 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



BEAM NO. 153 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

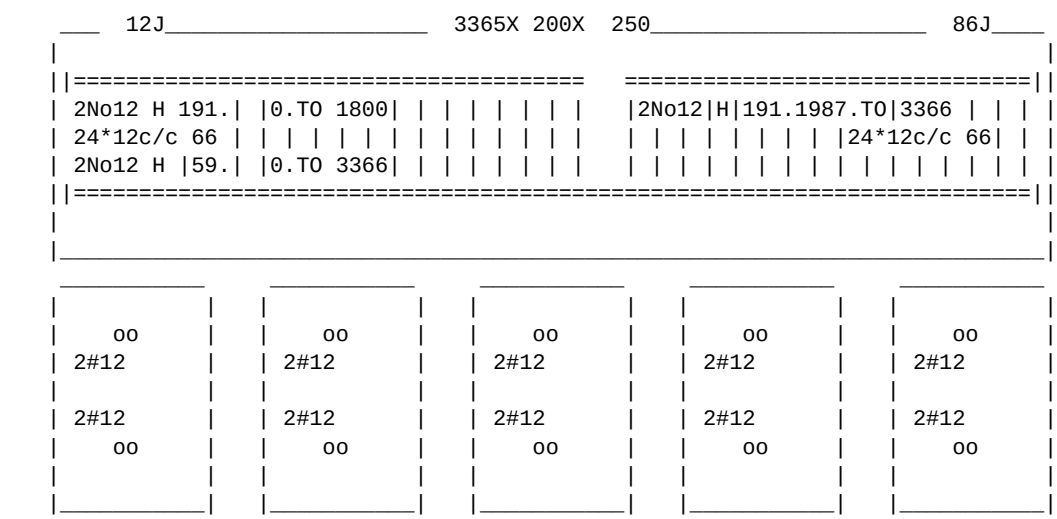
LEN - 3366. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3366.	YES	YES
2	191.	2 - 12MM	0.	1800.	YES	NO
3	191.	2 - 12MM	1987.	3366.	NO	YES

B E A M N O. 153 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 5.34 KNS Vc= 27.79 KNS Vs= 0.00 KNS
Tu= 1.04 KN-MET Tc= 0.7 KN-MET Ts= 1.4 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.60 SQ.CM.

AT END SUPPORT - Vu= 8.78 KNS Vc= 27.79 KNS Vs= 0.00 KNS
Tu= 1.04 KN-MET Tc= 0.7 KN-MET Ts= 1.4 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.60 SQ.CM.



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BEAM NO. 154 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

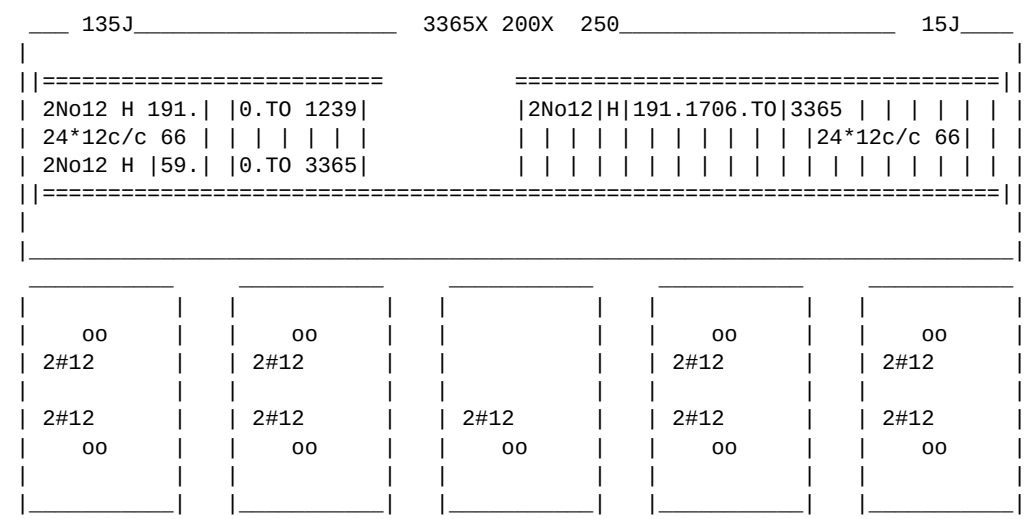
LEN - 3365. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3365.	YES	YES
2	191.	2 - 12MM	0.	1239.	YES	NO
3	191.	2 - 12MM	1706.	3365.	NO	YES

B E A M N O. 154 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 10.20 KNS Vc= 27.68 KNS Vs= 0.00 KNS
Tu= 1.06 KN-MET Tc= 0.7 KN-MET Ts= 1.4 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.62 SQ.CM.

AT END SUPPORT - Vu= 3.93 KNS Vc= 27.68 KNS Vs= 0.00 KNS
Tu= 1.06 KN-MET Tc= 0.7 KN-MET Ts= 1.4 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.62 SQ.CM.



BEAM NO. 155 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

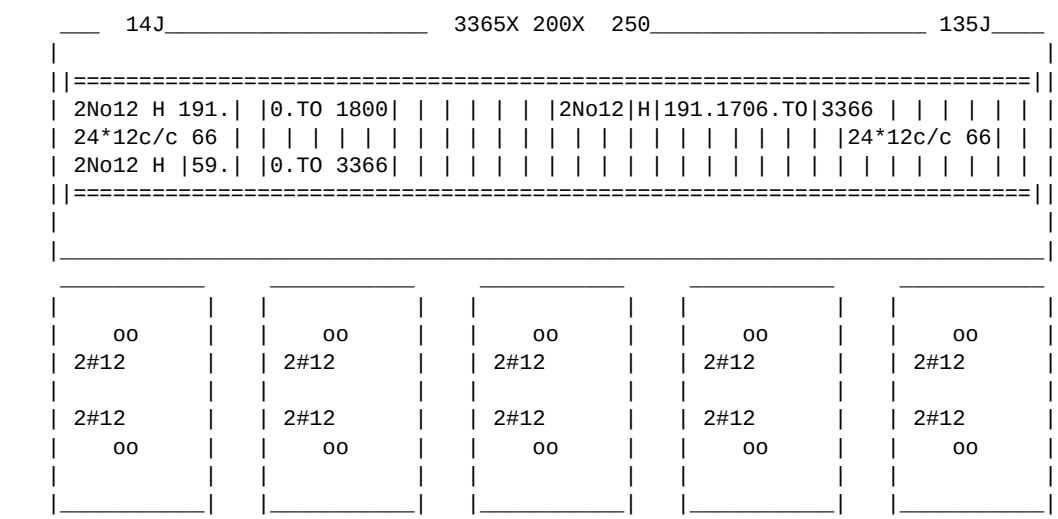
LEN - 3366. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3366.	YES	YES
2	191.	2 - 12MM	0.	1800.	YES	NO
3	191.	2 - 12MM	1706.	3366.	NO	YES

B E A M N O. 155 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 5.21 KNS Vc= 27.20 KNS Vs= 0.00 KNS
Tu= 0.92 KN-MET Tc= 0.7 KN-MET Ts= 1.2 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.54 SQ.CM.

AT END SUPPORT - Vu= 8.92 KNS Vc= 27.20 KNS Vs= 0.00 KNS
Tu= 0.92 KN-MET Tc= 0.7 KN-MET Ts= 1.2 KN-MET LOAD 7
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.54 SQ.CM.



BEAM NO. 160 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

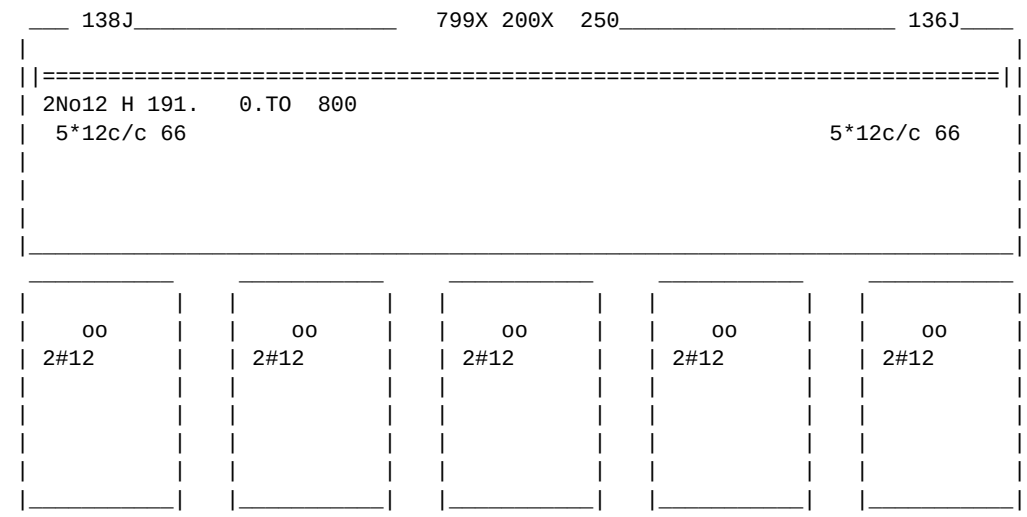
LEN - 800. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	800.	YES	YES

B E A M N O. 160 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 6.15 KNS Vc= 27.76 KNS Vs= 0.00 KNS
Tu= 1.35 KN-MET Tc= 0.8 KN-MET Ts= 1.8 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 215. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.78 SQ.CM.

AT END SUPPORT - Vu= 6.70 KNS Vc= 27.76 KNS Vs= 0.00 KNS
Tu= 1.35 KN-MET Tc= 0.8 KN-MET Ts= 1.8 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 215. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.78 SQ.CM.



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BEAM NO. 163 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

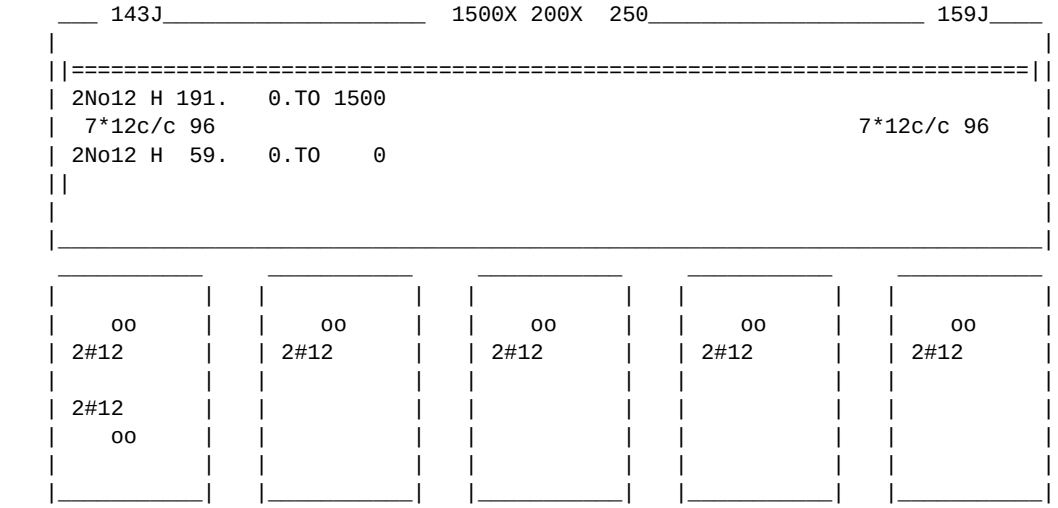
LEN - 1500. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	0.	YES	NO
2	191.	2 - 12MM	0.	1500.	YES	YES

B E A M N O. 163 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 4.48 KNS Vc= 28.92 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 565. MM

AT END SUPPORT - Vu= 5.97 KNS Vc= 28.82 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 565. MM



BEAM NO. 165 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

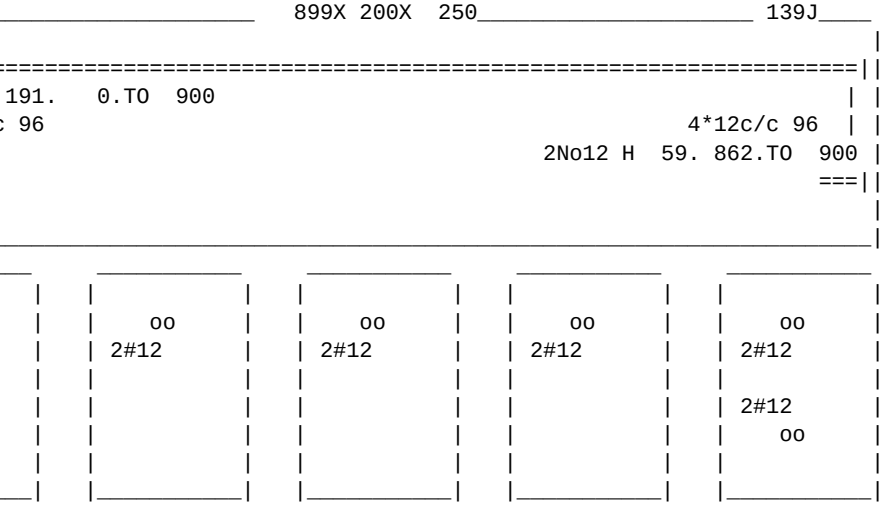
LEN - 900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	862.	900.	NO	YES
2	191.	2 - 12MM	0.	900.	YES	YES

B E A M N O. 165 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 5.14 KNS Vc= 28.78 KNS Vs= 0.00 KNS
Tu= 0.02 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 265. MM

AT END SUPPORT - Vu= 4.46 KNS Vc= 28.84 KNS Vs= 0.00 KNS
Tu= 0.02 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 265. MM



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BEAM NO. 166 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

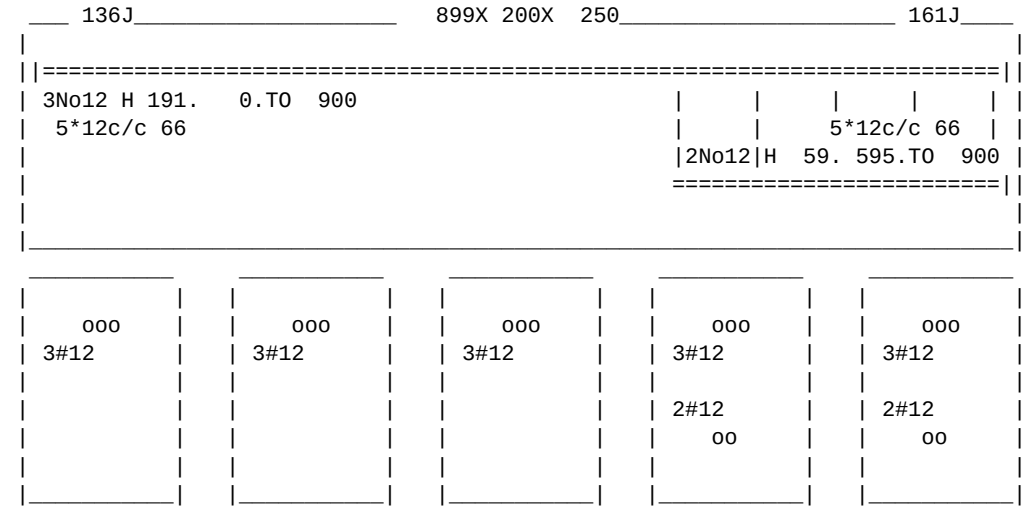
LEN - 900. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	595.	900.	NO	YES
2	191.	3 - 12MM	0.	900.	YES	YES

B E A M N O. 166 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 19.47 KNS Vc= 29.10 KNS Vs= 0.00 KNS
Tu= 1.88 KN-MET Tc= 0.8 KN-MET Ts= 2.5 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 265. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.09 SQ.CM.

AT END SUPPORT - Vu= 18.79 KNS Vc= 29.15 KNS Vs= 0.00 KNS
Tu= 1.88 KN-MET Tc= 0.8 KN-MET Ts= 2.5 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 265. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.09 SQ.CM.



BEAM NO. 167 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

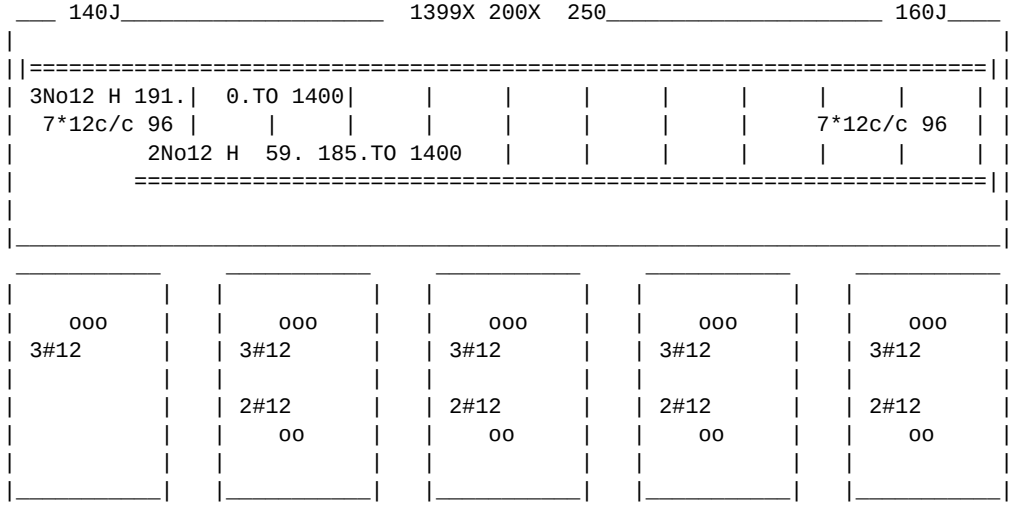
LEN - 1400. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	185.	1400.	NO	YES
2	191.	3 - 12MM	0.	1400.	YES	YES

B E A M N O. 167 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 16.41 KNS Vc= 28.50 KNS Vs= 0.00 KNS
Tu= 0.05 KN-MET Tc= 0.9 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 515. MM

AT END SUPPORT - Vu= 15.05 KNS Vc= 29.92 KNS Vs= 0.00 KNS
Tu= 0.05 KN-MET Tc= 0.9 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 515. MM



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BEAM NO. 169 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

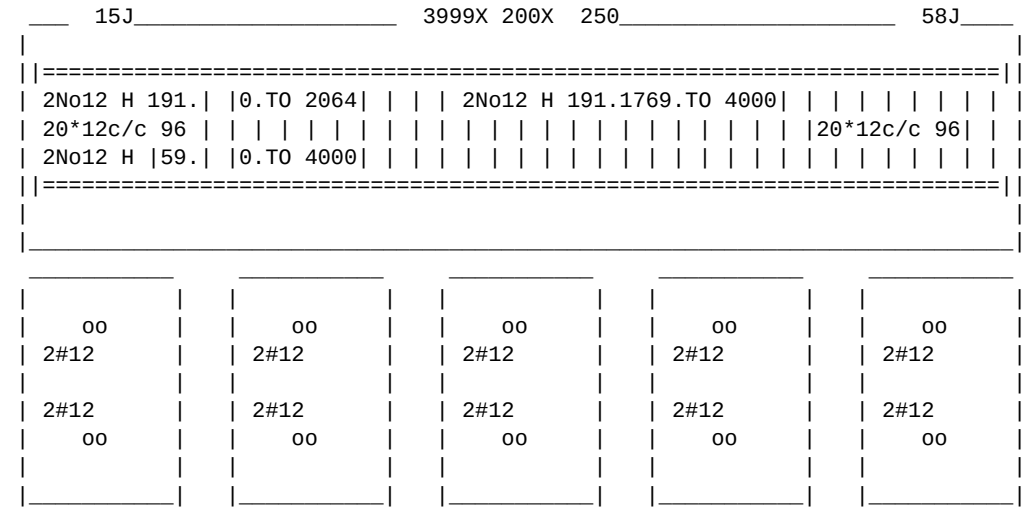
LEN - 4000. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	4000.	YES	YES
2	191.	2 - 12MM	0.	2064.	YES	NO
3	191.	2 - 12MM	1769.	4000.	NO	YES

B E A M N O. 169 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.55 KNS Vc= 29.07 KNS Vs= 0.00 KNS
Tu= 0.04 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1815. MM

AT END SUPPORT - Vu= 2.58 KNS Vc= 29.07 KNS Vs= 0.00 KNS
Tu= 0.04 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1815. MM



BEAM NO. 171 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

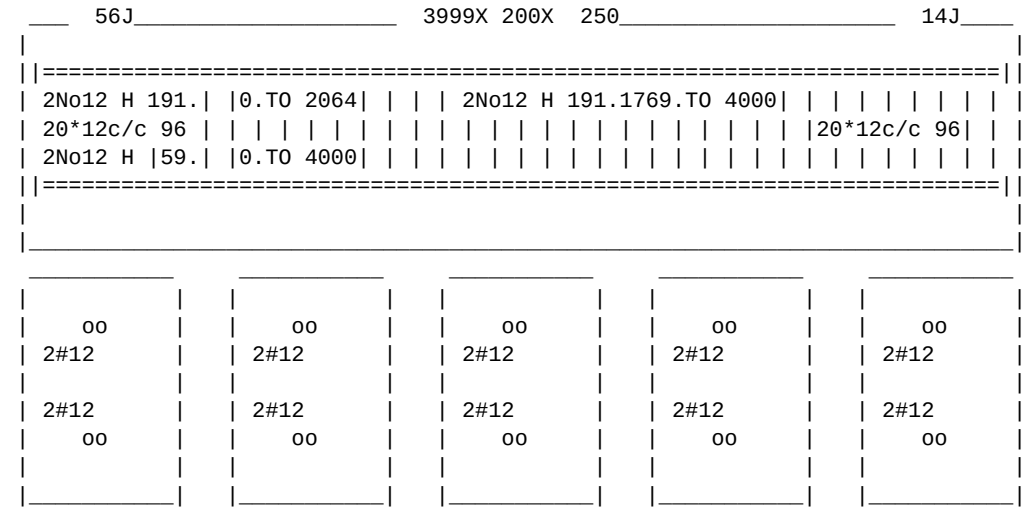
LEN - 4000. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	4000.	YES	YES
2	191.	2 - 12MM	0.	2064.	YES	NO
3	191.	2 - 12MM	1769.	4000.	NO	YES

B E A M N O. 171 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.59 KNS Vc= 29.06 KNS Vs= 0.00 KNS
Tu= 0.19 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1815. MM

AT END SUPPORT - Vu= 2.55 KNS Vc= 29.06 KNS Vs= 0.00 KNS
Tu= 0.19 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 1815. MM



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BEAM NO. 173 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

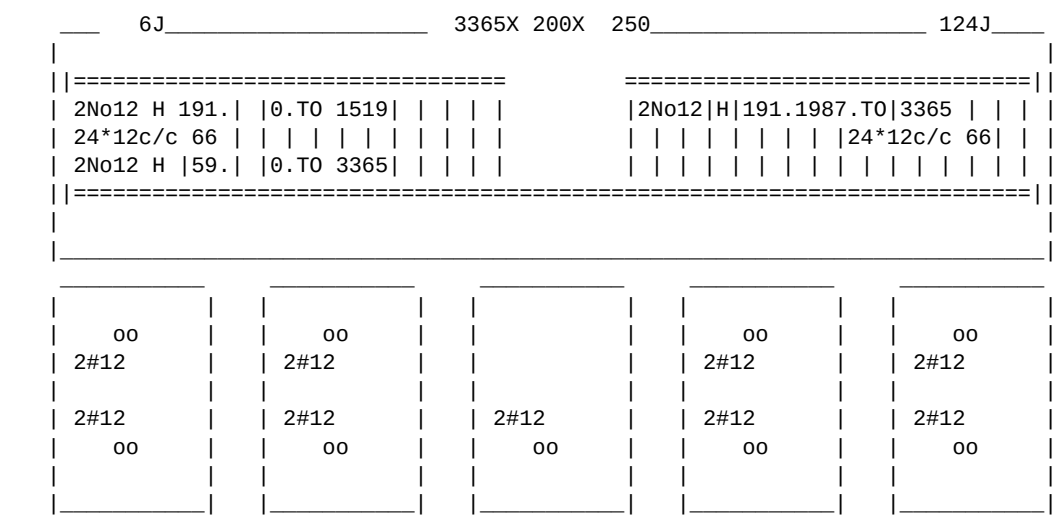
LEN - 3365. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	3365.	YES	YES
2	191.	2 - 12MM	0.	1519.	YES	NO
3	191.	2 - 12MM	1987.	3365.	NO	YES

B E A M N O. 173 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 7.13 KNS Vc= 27.50 KNS Vs= 0.00 KNS
Tu= 1.00 KN-MET Tc= 0.7 KN-MET Ts= 1.3 KN-MET LOAD 8
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.58 SQ.CM.

AT END SUPPORT - Vu= 6.99 KNS Vc= 27.50 KNS Vs= 0.00 KNS
Tu= 1.00 KN-MET Tc= 0.7 KN-MET Ts= 1.3 KN-MET LOAD 8
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 1498. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.58 SQ.CM.



BEAM NO. 177 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

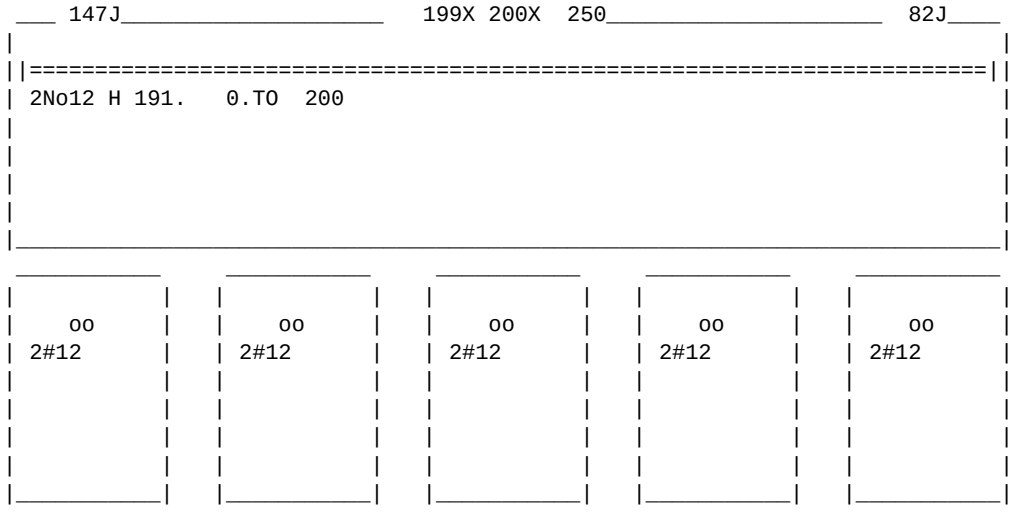
LEN - 200. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	200.	YES	YES

B E A M N O. 177 D E S I G N R E S U L T S - SHEAR

** LOCATION FOR DESIGN FOR SHEAR AT START OF MEMBER 177 IS BEYOND
THE MIDPOINT OF MEMBER. DESIGN FOR SHEAR AND TORSION NOT PERFORMED.

** LOCATION FOR DESIGN FOR SHEAR AT END OF MEMBER 177 IS BEYOND
THE MIDPOINT OF MEMBER. DESIGN FOR SHEAR AND TORSION NOT PERFORMED.



BEAM NO. 178 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

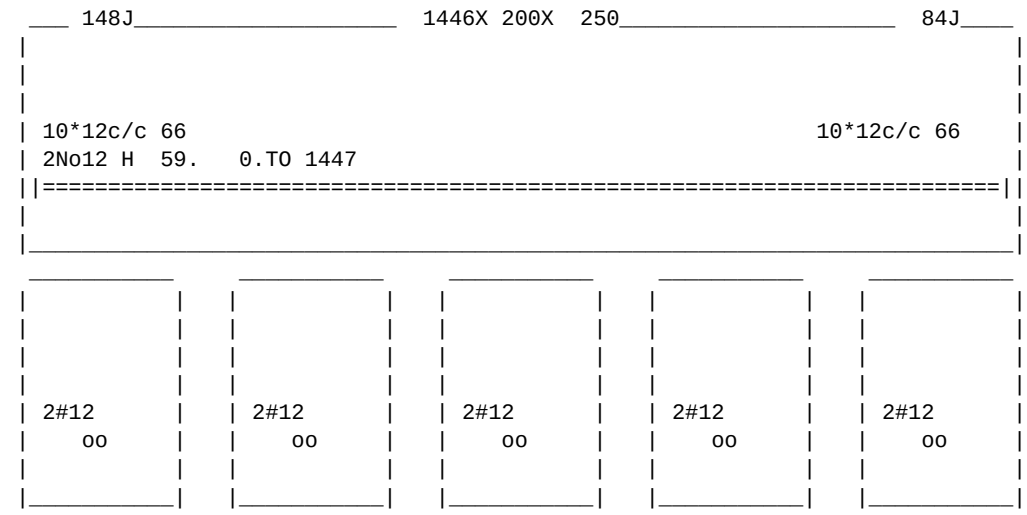
LEN - 1447. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1447.	YES	YES

B E A M N O. 178 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 1.62 KNS Vc= 27.89 KNS Vs= 0.00 KNS
Tu= 0.86 KN-MET Tc= 0.8 KN-MET Ts= 1.1 KN-MET LOAD 11
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 539. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.50 SQ.CM.

AT END SUPPORT - Vu= 3.05 KNS Vc= 28.56 KNS Vs= 0.00 KNS
Tu= 0.86 KN-MET Tc= 0.8 KN-MET Ts= 1.1 KN-MET LOAD 11
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 539. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.50 SQ.CM.



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BEAM NO. 179 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

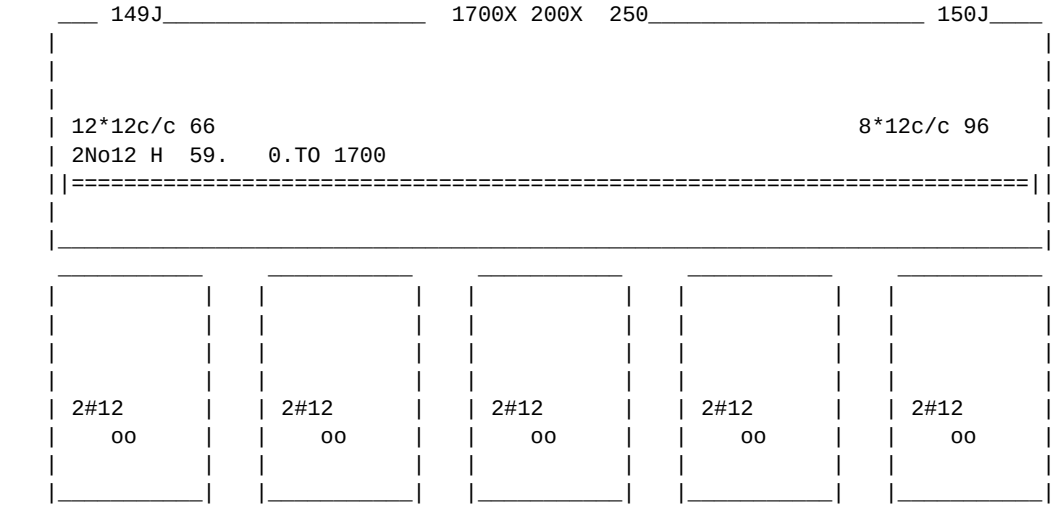
LEN - 1700. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1700.	YES	YES

B E A M N O. 179 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 1.27 KNS Vc= 28.21 KNS Vs= 0.00 KNS
Tu= 0.83 KN-MET Tc= 0.8 KN-MET Ts= 1.1 KN-MET LOAD 10
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 665. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.48 SQ.CM.

AT END SUPPORT - Vu= 0.53 KNS Vc= 27.81 KNS Vs= 0.00 KNS
Tu= 0.51 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 665. MM



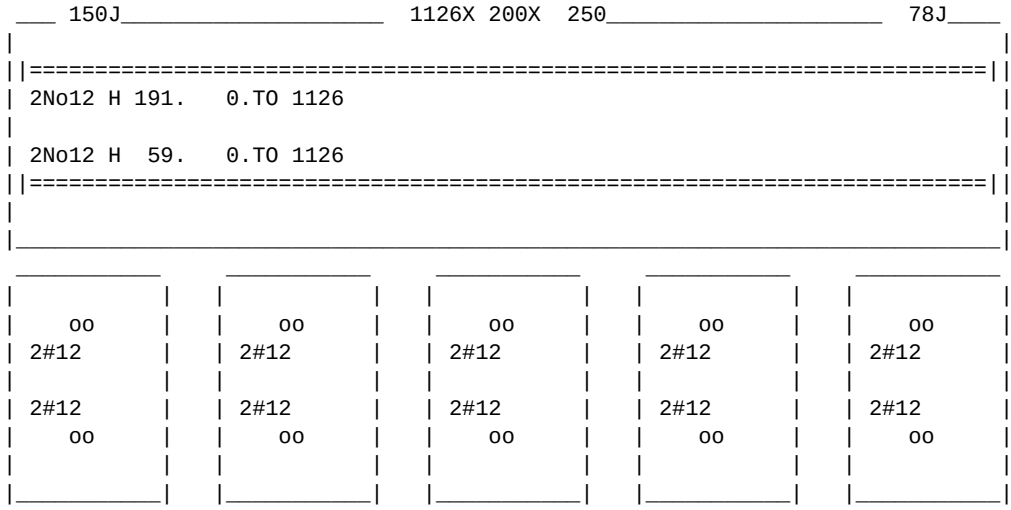
BEAM NO. 180 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 1126. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1126.	YES	YES
2	191.	2 - 12MM	0.	1126.	YES	YES

B E A M N O. 180 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 7.27 KNS Tu= 4.2 KN-MET
Vc= 29.1 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 8.23 KNS Tu= 4.2 KN-MET
Vc= 29.0 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



BEAM NO. 181 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

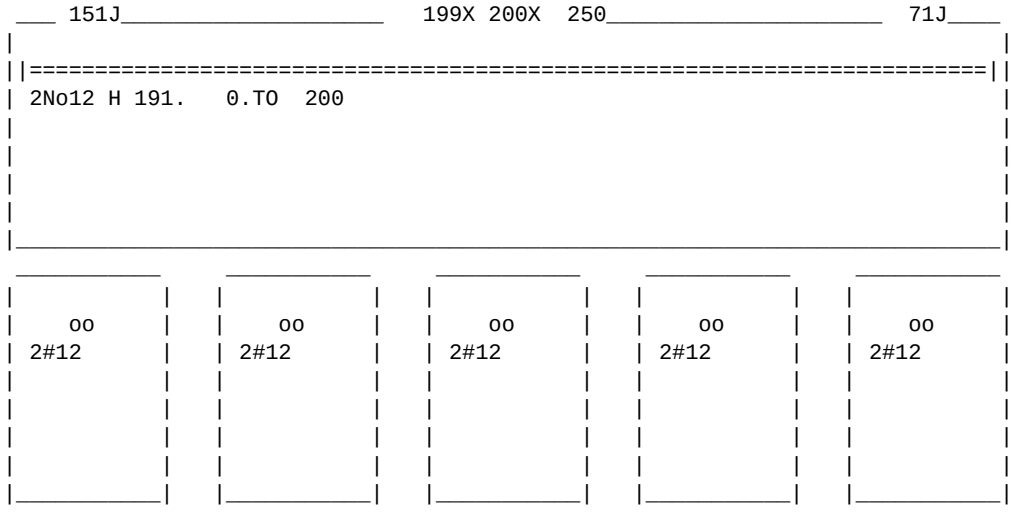
LEN - 200. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	200.	YES	YES

B E A M N O. 181 D E S I G N R E S U L T S - SHEAR

** LOCATION FOR DESIGN FOR SHEAR AT START OF MEMBER 181 IS BEYOND
THE MIDPOINT OF MEMBER. DESIGN FOR SHEAR AND TORSION NOT PERFORMED.

** LOCATION FOR DESIGN FOR SHEAR AT END OF MEMBER 181 IS BEYOND
THE MIDPOINT OF MEMBER. DESIGN FOR SHEAR AND TORSION NOT PERFORMED.



BEAM NO. 182 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

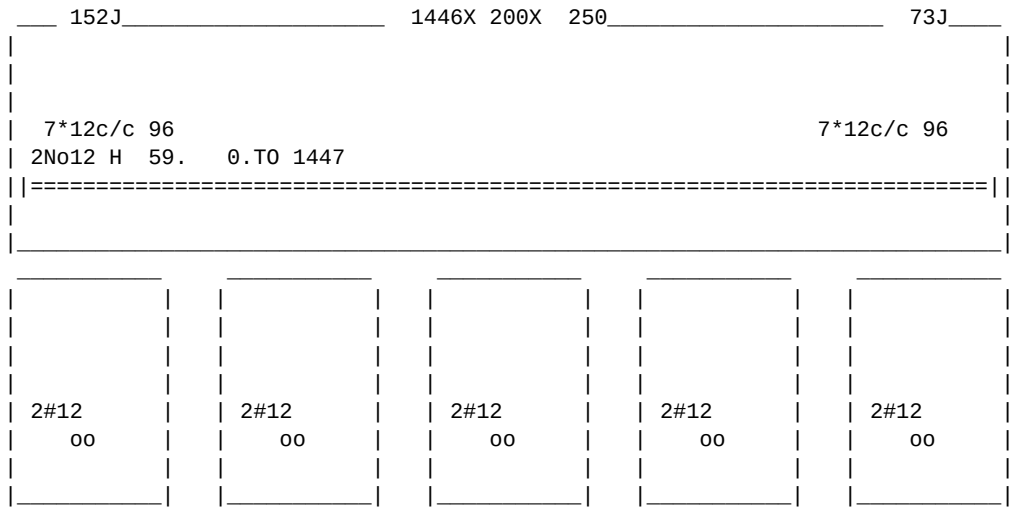
LEN - 1447. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1447.	YES	YES

B E A M N O. 182 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.45 KNS Vc= 27.97 KNS Vs= 0.00 KNS
Tu= 0.04 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 539. MM

AT END SUPPORT - Vu= 3.88 KNS Vc= 28.73 KNS Vs= 0.00 KNS
Tu= 0.04 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 539. MM



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BEAM NO. 183 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

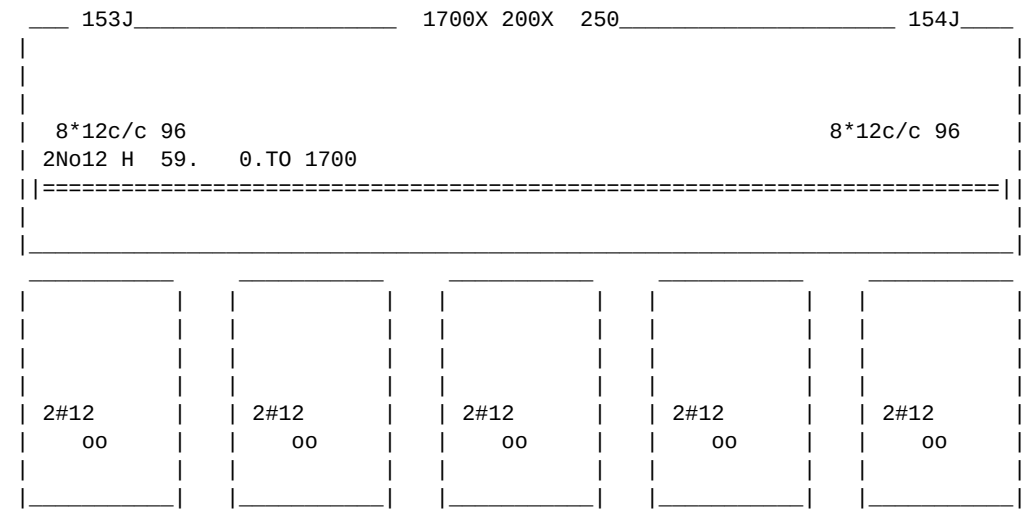
LEN - 1700. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1700.	YES	YES

B E A M N O. 183 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.95 KNS Vc= 28.16 KNS Vs= 0.00 KNS
Tu= 0.36 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 665. MM

AT END SUPPORT - Vu= 1.25 KNS Vc= 27.85 KNS Vs= 0.00 KNS
Tu= 0.36 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 665. MM



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BEAM NO. 184 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

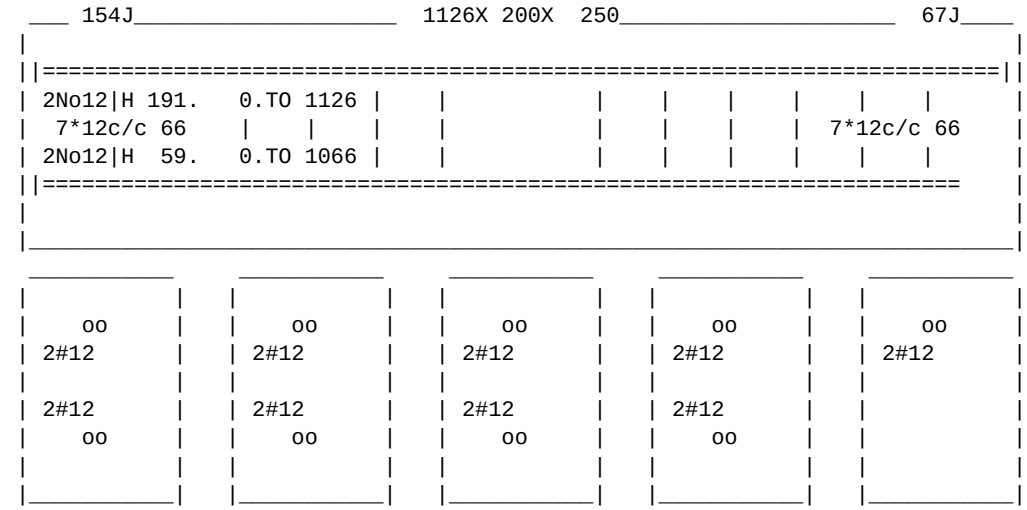
LEN - 1126. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1066.	YES	NO
2	191.	2 - 12MM	0.	1126.	YES	YES

B E A M N O. 184 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 11.00 KNS Vc= 30.83 KNS Vs= 0.00 KNS
Tu= 2.46 KN-MET Tc= 0.9 KN-MET Ts= 3.3 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 379. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.43 SQ.CM.

AT END SUPPORT - Vu= 11.97 KNS Vc= 28.98 KNS Vs= 0.00 KNS
Tu= 2.46 KN-MET Tc= 0.9 KN-MET Ts= 3.3 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 379. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.43 SQ.CM.



BEAM NO. 185 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

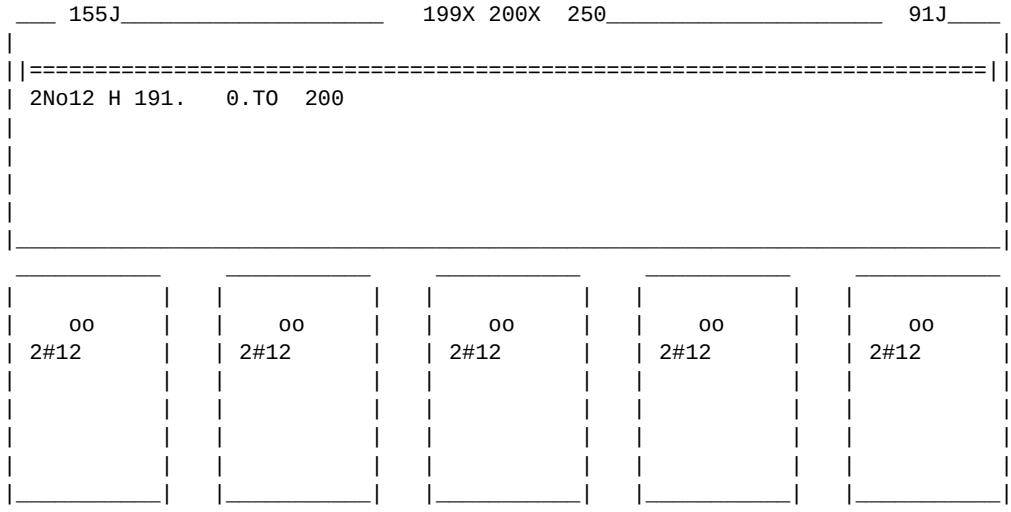
LEN - 200. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	200.	YES	YES

B E A M N O. 185 D E S I G N R E S U L T S - SHEAR

** LOCATION FOR DESIGN FOR SHEAR AT START OF MEMBER 185 IS BEYOND
THE MIDPOINT OF MEMBER. DESIGN FOR SHEAR AND TORSION NOT PERFORMED.

** LOCATION FOR DESIGN FOR SHEAR AT END OF MEMBER 185 IS BEYOND
THE MIDPOINT OF MEMBER. DESIGN FOR SHEAR AND TORSION NOT PERFORMED.



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BEAM NO. 186 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

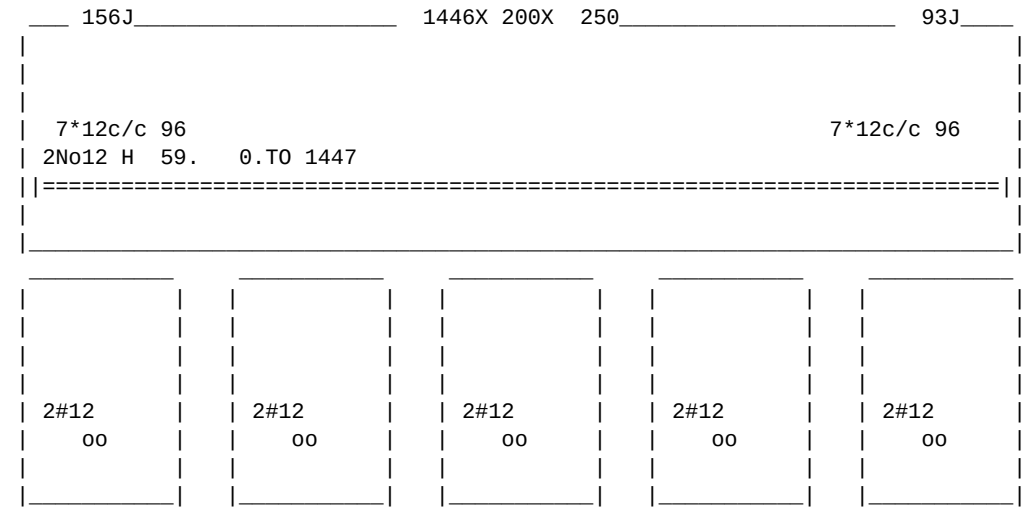
LEN - 1447. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1447.	YES	YES

B E A M N O. 186 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.45 KNS Vc= 27.97 KNS Vs= 0.00 KNS
Tu= 0.02 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 539. MM

AT END SUPPORT - Vu= 3.88 KNS Vc= 28.73 KNS Vs= 0.00 KNS
Tu= 0.02 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 539. MM



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BEAM NO. 187 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

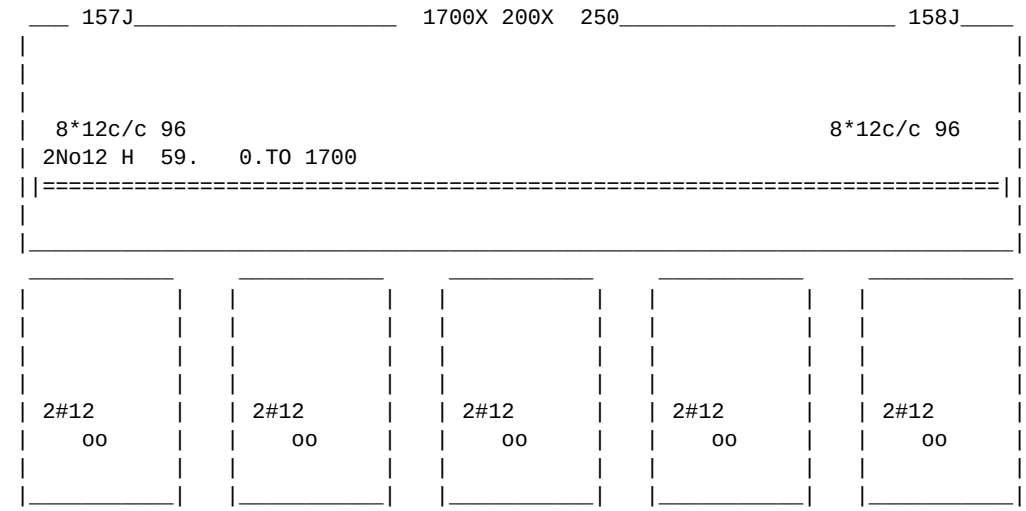
LEN - 1700. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1700.	YES	YES

B E A M N O. 187 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.97 KNS Vc= 28.16 KNS Vs= 0.00 KNS
Tu= 0.40 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 665. MM

AT END SUPPORT - Vu= 1.27 KNS Vc= 27.85 KNS Vs= 0.00 KNS
Tu= 0.40 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 665. MM



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BEAM NO. 188 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

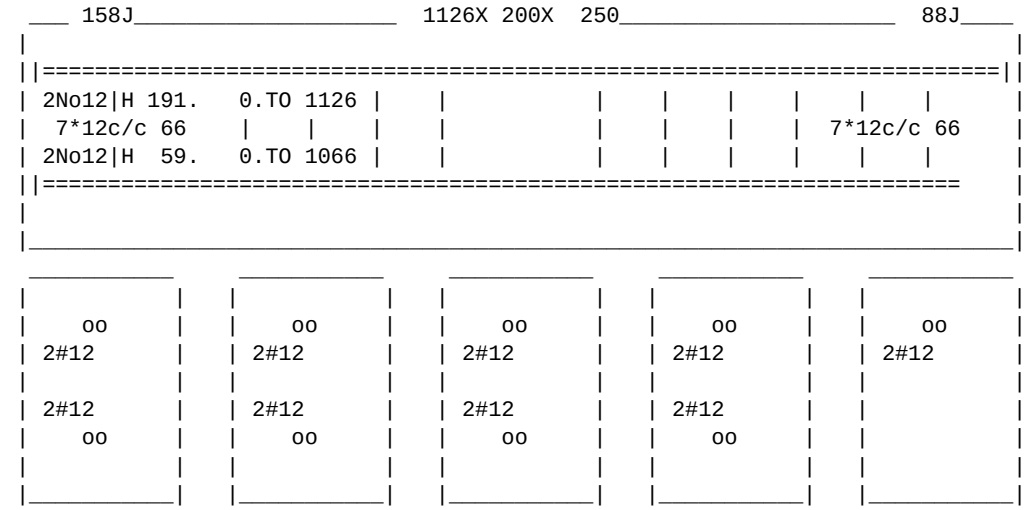
LEN - 1126. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1066.	YES	NO
2	191.	2 - 12MM	0.	1126.	YES	YES

B E A M N O. 188 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 10.92 KNS Vc= 29.20 KNS Vs= 0.00 KNS
Tu= 2.50 KN-MET Tc= 0.9 KN-MET Ts= 3.3 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 379. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.46 SQ.CM.

AT END SUPPORT - Vu= 11.88 KNS Vc= 29.50 KNS Vs= 0.00 KNS
Tu= 2.50 KN-MET Tc= 0.9 KN-MET Ts= 3.3 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 379. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.46 SQ.CM.



BEAM NO. 189 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

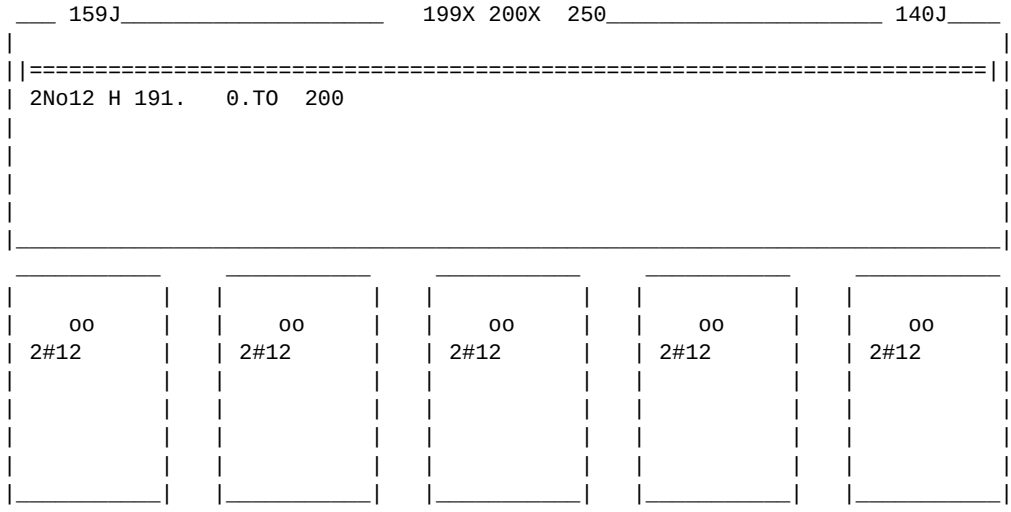
LEN - 200. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	200.	YES	YES

B E A M N O. 189 D E S I G N R E S U L T S - SHEAR

** LOCATION FOR DESIGN FOR SHEAR AT START OF MEMBER 189 IS BEYOND
THE MIDPOINT OF MEMBER. DESIGN FOR SHEAR AND TORSION NOT PERFORMED.

** LOCATION FOR DESIGN FOR SHEAR AT END OF MEMBER 189 IS BEYOND
THE MIDPOINT OF MEMBER. DESIGN FOR SHEAR AND TORSION NOT PERFORMED.



BEAM NO. 190 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

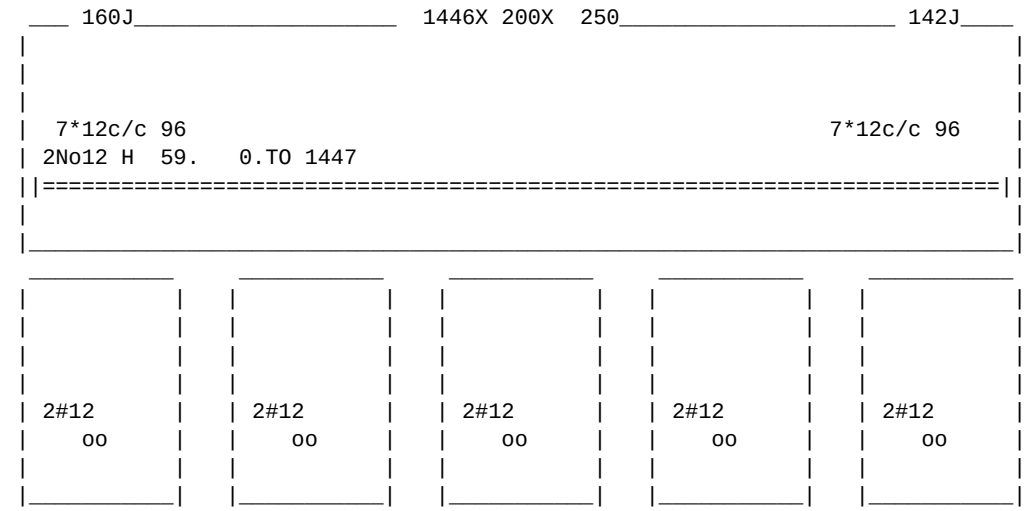
LEN - 1447. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1447.	YES	YES

B E A M N O. 190 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.06 KNS Vc= 27.91 KNS Vs= 0.00 KNS
Tu= 0.08 KN-MET Tc= 0.9 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 539. MM

AT END SUPPORT - Vu= 3.49 KNS Vc= 28.24 KNS Vs= 0.00 KNS
Tu= 0.08 KN-MET Tc= 0.9 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 539. MM



BEAM NO. 191 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

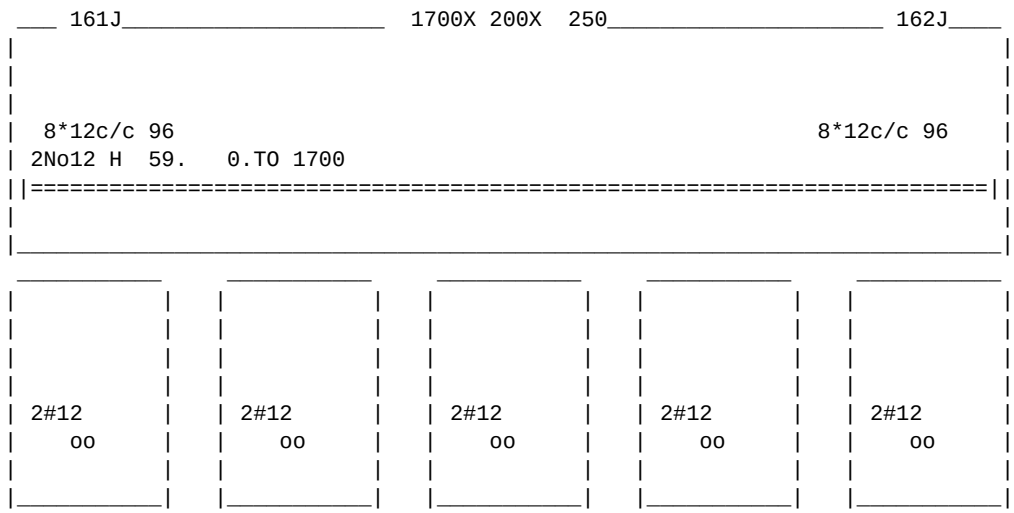
LEN - 1700. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1700.	YES	YES

B E A M N O. 191 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 3.33 KNS Vc= 28.19 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 665. MM

AT END SUPPORT - Vu= 1.63 KNS Vc= 27.87 KNS Vs= 0.00 KNS
Tu= 0.01 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 665. MM



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BEAM NO. 192 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

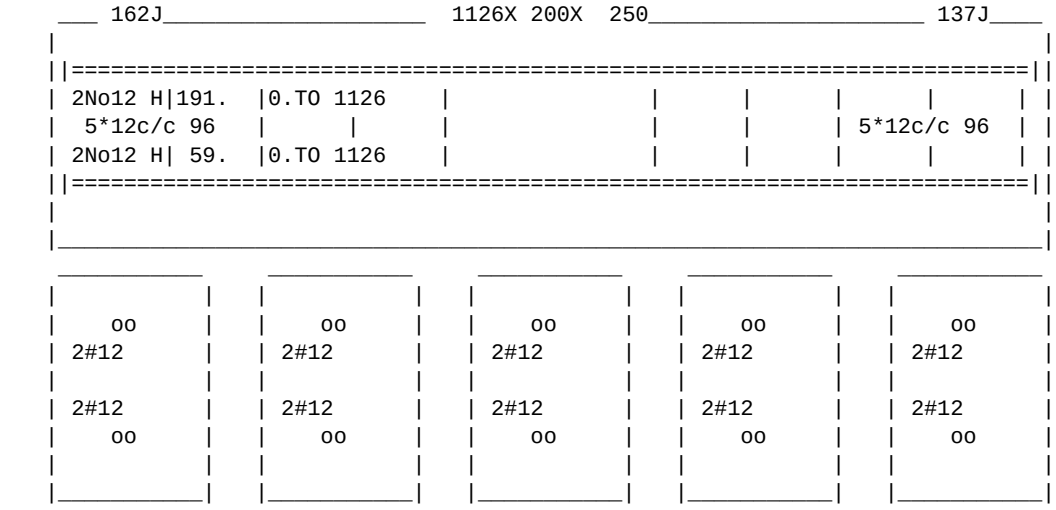
LEN - 1126. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1126.	YES	YES
2	191.	2 - 12MM	0.	1126.	YES	YES

B E A M N O. 192 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 14.85 KNS Vc= 29.69 KNS Vs= 0.00 KNS
Tu= 0.10 KN-MET Tc= 0.9 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 379. MM

AT END SUPPORT - Vu= 15.82 KNS Vc= 29.20 KNS Vs= 0.00 KNS
Tu= 0.10 KN-MET Tc= 0.9 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 379. MM



BEAM NO. 193 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

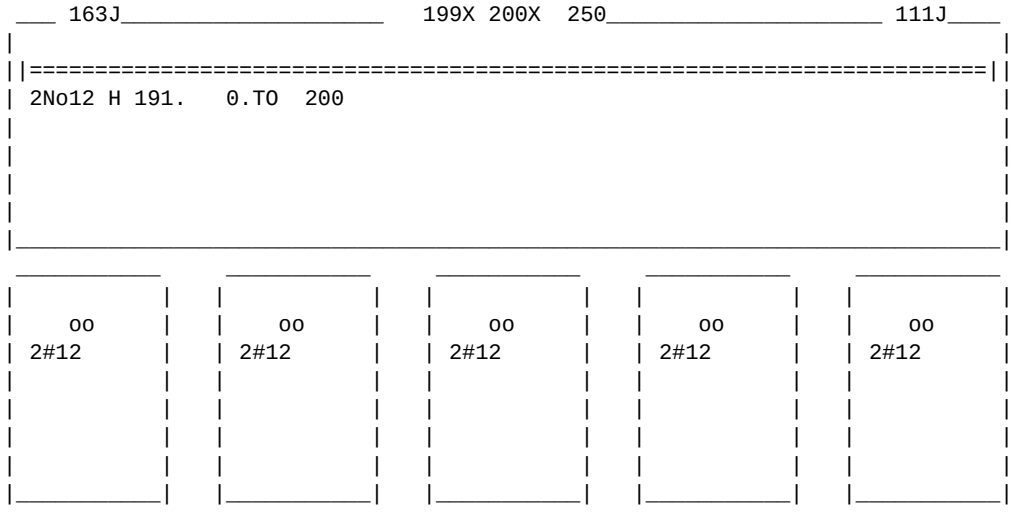
LEN - 200. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	200.	YES	YES

B E A M N O. 193 D E S I G N R E S U L T S - SHEAR

** LOCATION FOR DESIGN FOR SHEAR AT START OF MEMBER 193 IS BEYOND
THE MIDPOINT OF MEMBER. DESIGN FOR SHEAR AND TORSION NOT PERFORMED.

** LOCATION FOR DESIGN FOR SHEAR AT END OF MEMBER 193 IS BEYOND
THE MIDPOINT OF MEMBER. DESIGN FOR SHEAR AND TORSION NOT PERFORMED.



BEAM NO. 194 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

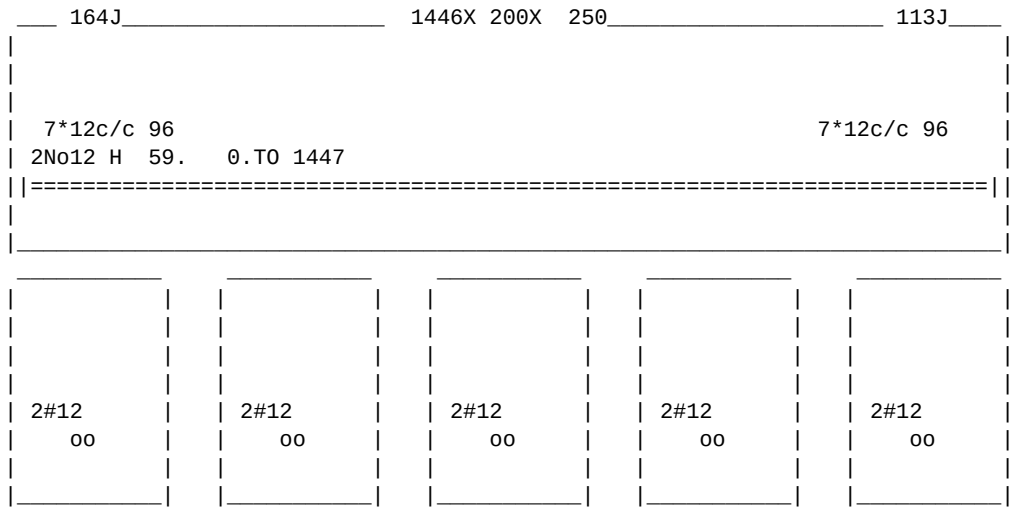
LEN - 1447. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1447.	YES	YES

B E A M N O. 194 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.40 KNS Vc= 27.96 KNS Vs= 0.00 KNS
Tu= 0.05 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 539. MM

AT END SUPPORT - Vu= 3.83 KNS Vc= 28.72 KNS Vs= 0.00 KNS
Tu= 0.05 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 539. MM



BEAM NO. 195 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

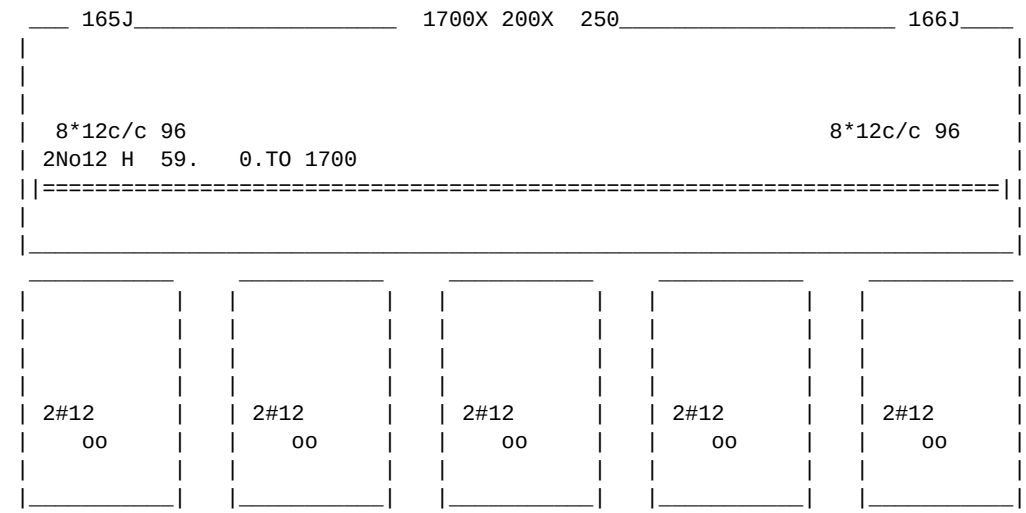
LEN - 1700. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1700.	YES	YES

B E A M N O. 195 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.96 KNS Vc= 28.16 KNS Vs= 0.00 KNS
Tu= 0.41 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 665. MM

AT END SUPPORT - Vu= 1.26 KNS Vc= 27.85 KNS Vs= 0.00 KNS
Tu= 0.41 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 665. MM



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BEAM NO. 196 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

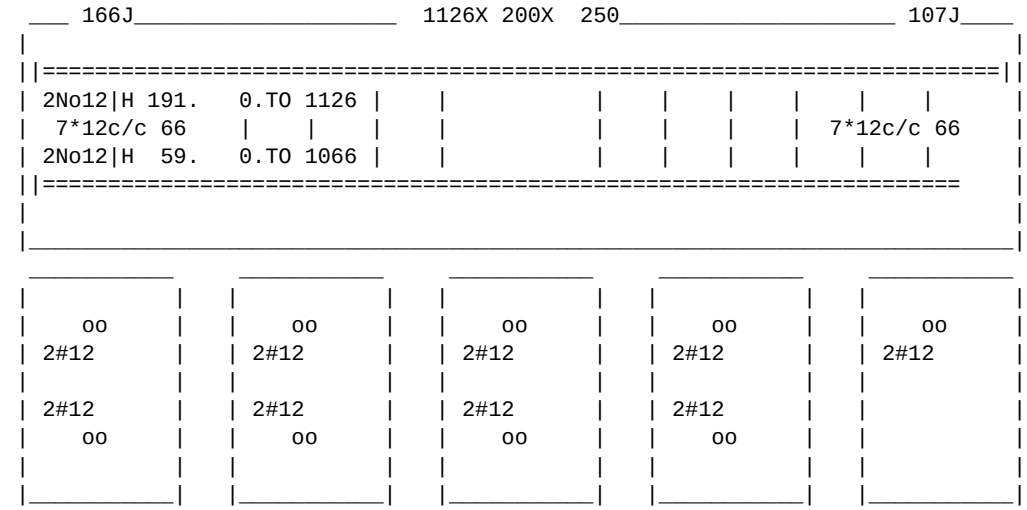
LEN - 1126. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1066.	YES	NO
2	191.	2 - 12MM	0.	1126.	YES	YES

B E A M N O. 196 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 11.00 KNS Vc= 30.83 KNS Vs= 0.00 KNS
Tu= 2.39 KN-MET Tc= 0.9 KN-MET Ts= 3.2 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 379. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.39 SQ.CM.

AT END SUPPORT - Vu= 11.96 KNS Vc= 28.97 KNS Vs= 0.00 KNS
Tu= 2.39 KN-MET Tc= 0.9 KN-MET Ts= 3.2 KN-MET LOAD 6
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 379. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.39 SQ.CM.



BEAM NO. 197 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

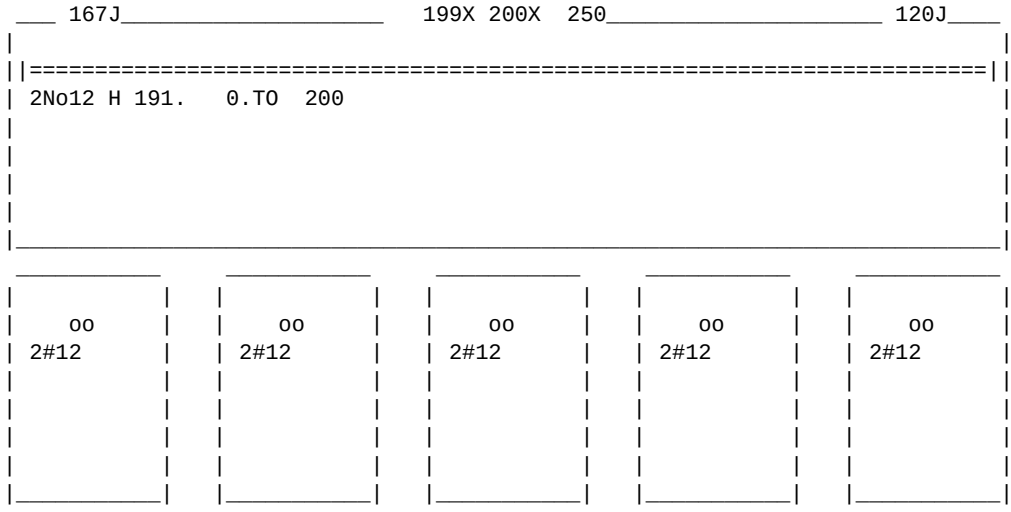
LEN - 200. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	200.	YES	YES

B E A M N O. 197 D E S I G N R E S U L T S - SHEAR

** LOCATION FOR DESIGN FOR SHEAR AT START OF MEMBER 197 IS BEYOND
THE MIDPOINT OF MEMBER. DESIGN FOR SHEAR AND TORSION NOT PERFORMED.

** LOCATION FOR DESIGN FOR SHEAR AT END OF MEMBER 197 IS BEYOND
THE MIDPOINT OF MEMBER. DESIGN FOR SHEAR AND TORSION NOT PERFORMED.



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BEAM NO. 198 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

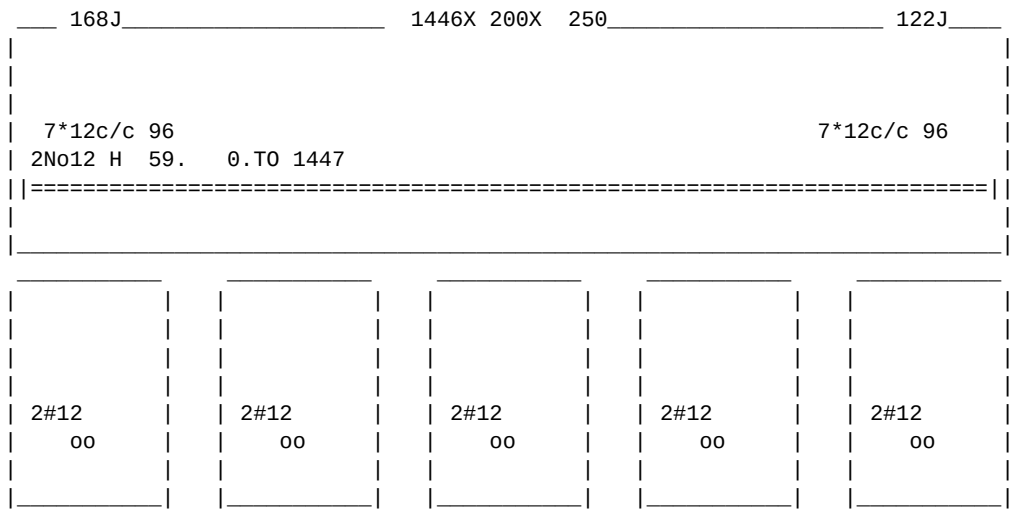
LEN - 1447. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1447.	YES	YES

B E A M N O. 198 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.45 KNS Vc= 27.96 KNS Vs= 0.00 KNS
Tu= 0.06 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 539. MM

AT END SUPPORT - Vu= 3.88 KNS Vc= 28.73 KNS Vs= 0.00 KNS
Tu= 0.06 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 539. MM



BEAM NO. 199 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

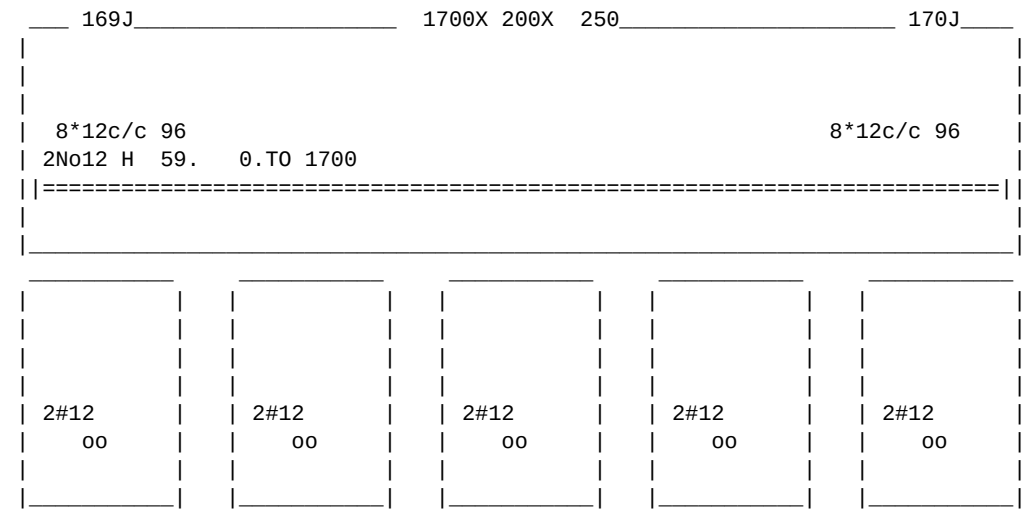
LEN - 1700. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1700.	YES	YES

B E A M N O. 199 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.95 KNS Vc= 28.16 KNS Vs= 0.00 KNS
Tu= 0.36 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 665. MM

AT END SUPPORT - Vu= 1.25 KNS Vc= 27.85 KNS Vs= 0.00 KNS
Tu= 0.36 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 665. MM



BEAM NO. 200 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

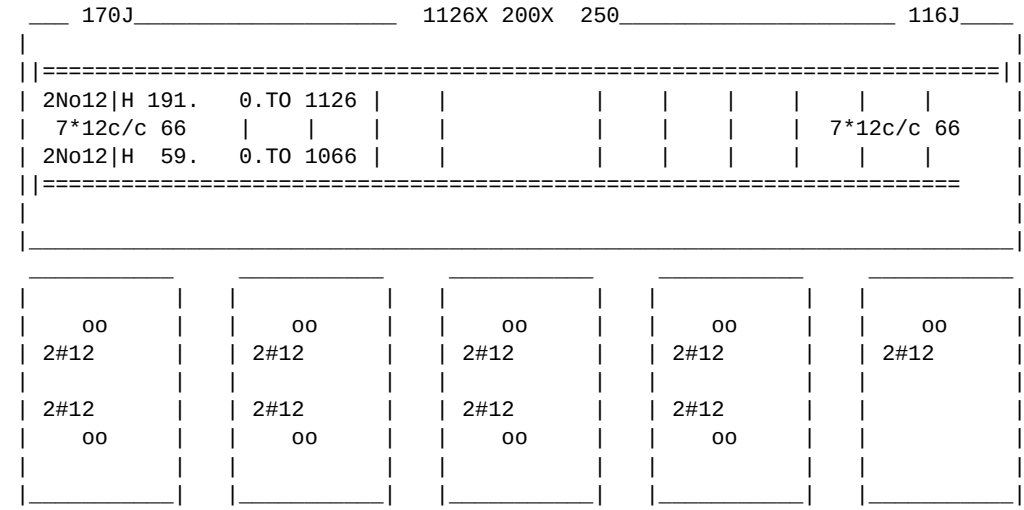
LEN - 1126. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1066.	YES	NO
2	191.	2 - 12MM	0.	1126.	YES	YES

B E A M N O. 200 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 10.80 KNS Vc= 29.20 KNS Vs= 0.00 KNS
Tu= 2.47 KN-MET Tc= 0.9 KN-MET Ts= 3.3 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 379. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.44 SQ.CM.

AT END SUPPORT - Vu= 11.77 KNS Vc= 29.50 KNS Vs= 0.00 KNS
Tu= 2.47 KN-MET Tc= 0.9 KN-MET Ts= 3.3 KN-MET LOAD 9
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 379. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 1.44 SQ.CM.



BEAM NO. 201 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

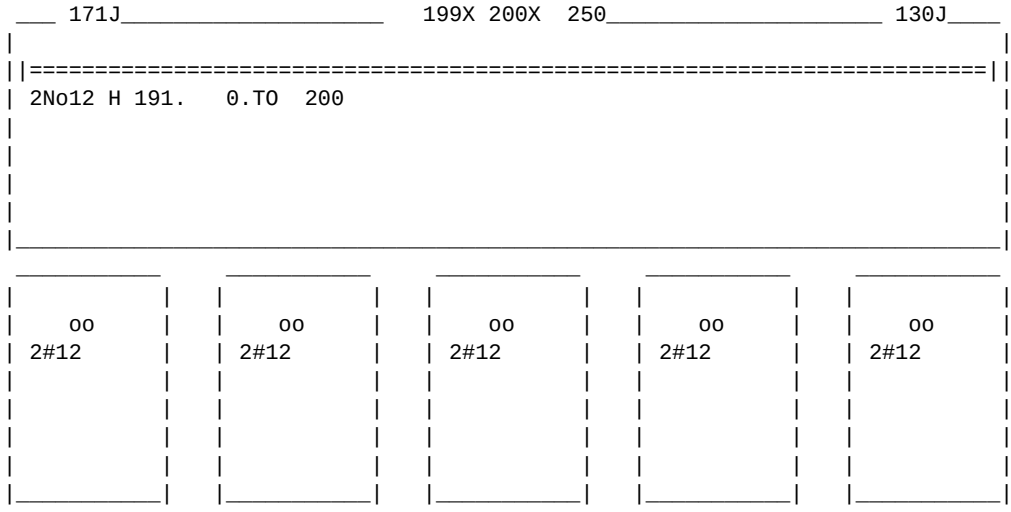
LEN - 200. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	191.	2 - 12MM	0.	200.	YES	YES

B E A M N O. 201 D E S I G N R E S U L T S - SHEAR

** LOCATION FOR DESIGN FOR SHEAR AT START OF MEMBER 201 IS BEYOND
THE MIDPOINT OF MEMBER. DESIGN FOR SHEAR AND TORSION NOT PERFORMED.

** LOCATION FOR DESIGN FOR SHEAR AT END OF MEMBER 201 IS BEYOND
THE MIDPOINT OF MEMBER. DESIGN FOR SHEAR AND TORSION NOT PERFORMED.



BEAM NO. 202 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

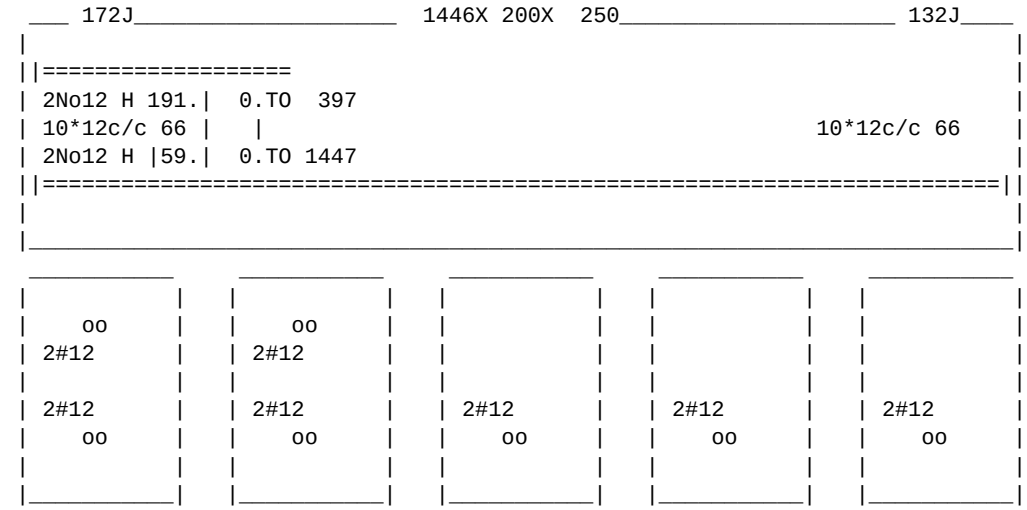
LEN - 1447. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1447.	YES	YES
2	191.	2 - 12MM	0.	397.	YES	NO

B E A M N O. 202 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 0.77 KNS Vc= 28.39 KNS Vs= 0.00 KNS
Tu= 1.07 KN-MET Tc= 0.9 KN-MET Ts= 1.4 KN-MET LOAD 12
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 539. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.62 SQ.CM.

AT END SUPPORT - Vu= 0.66 KNS Vc= 28.17 KNS Vs= 0.00 KNS
Tu= 1.07 KN-MET Tc= 0.8 KN-MET Ts= 1.4 KN-MET LOAD 12
STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 66. MM C/C FOR 539. MM
ADDITIONAL LONGITUDINAL STEEL REQD. FOR TORSIONAL RESISTANCE = 0.62 SQ.CM.



BEAM NO. 203 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

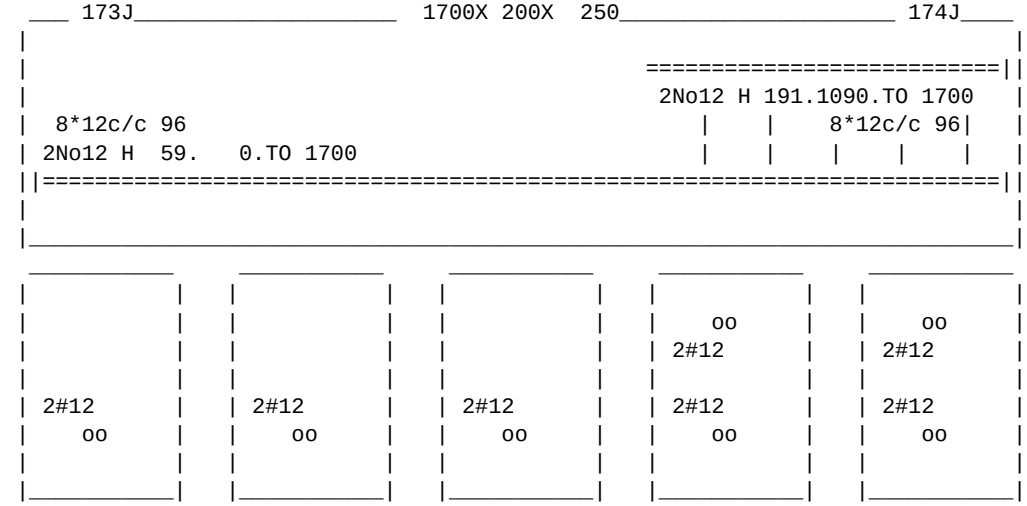
LEN - 1700. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1700.	YES	YES
2	191.	2 - 12MM	1090.	1700.	NO	YES

B E A M N O. 203 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 2.24 KNS Vc= 28.28 KNS Vs= 0.00 KNS
Tu= 0.50 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 665. MM

AT END SUPPORT - Vu= 0.54 KNS Vc= 27.82 KNS Vs= 0.00 KNS
Tu= 0.50 KN-MET Tc= 0.8 KN-MET Ts= 0.0 KN-MET LOAD 5
NO STIRRUPS ARE REQUIRED FOR TORSION.
REINFORCEMENT FOR SHEAR IS PER CL.11.5.5.1.
PROVIDE 12 MM 2-LEGGED STIRRUPS AT 96. MM C/C FOR 665. MM



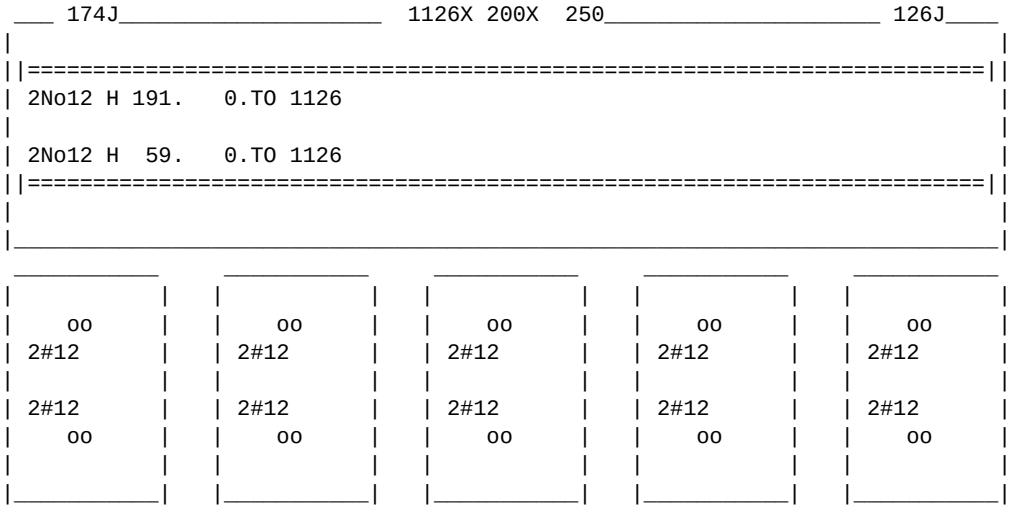
BEAM NO. 204 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 1126. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	1126.	YES	YES
2	191.	2 - 12MM	0.	1126.	YES	YES

B E A M N O. 204 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 7.37 KNS Tu= 4.4 KN-MET
Vc= 29.3 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 8.34 KNS Tu= 4.4 KN-MET
Vc= 29.2 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



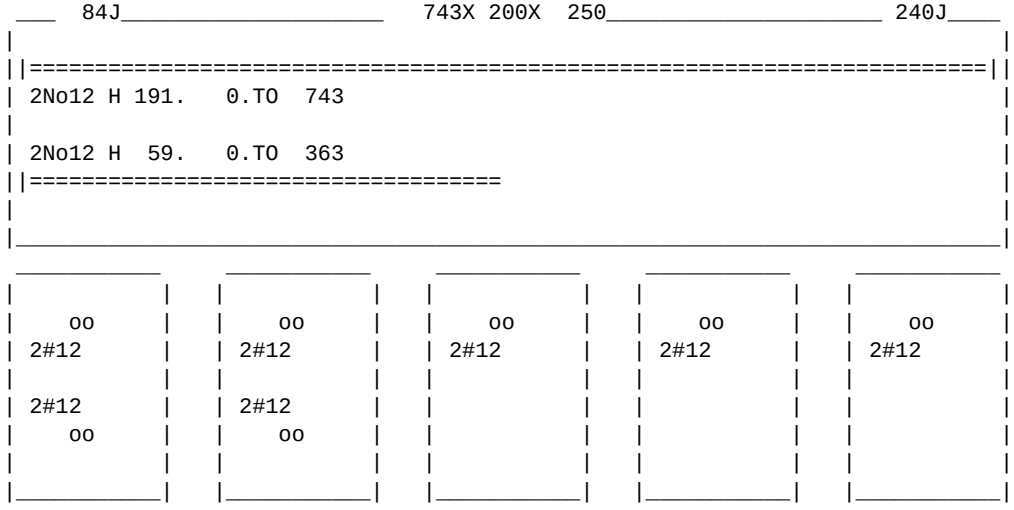
BEAM NO. 360 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 743. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	363.	YES	NO
2	191.	2 - 12MM	0.	743.	YES	YES

B E A M N O. 360 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 8.35 KNS Tu= 4.2 KN-MET
Vc= 51.9 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 8.84 KNS Tu= 4.2 KN-MET
Vc= 28.1 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



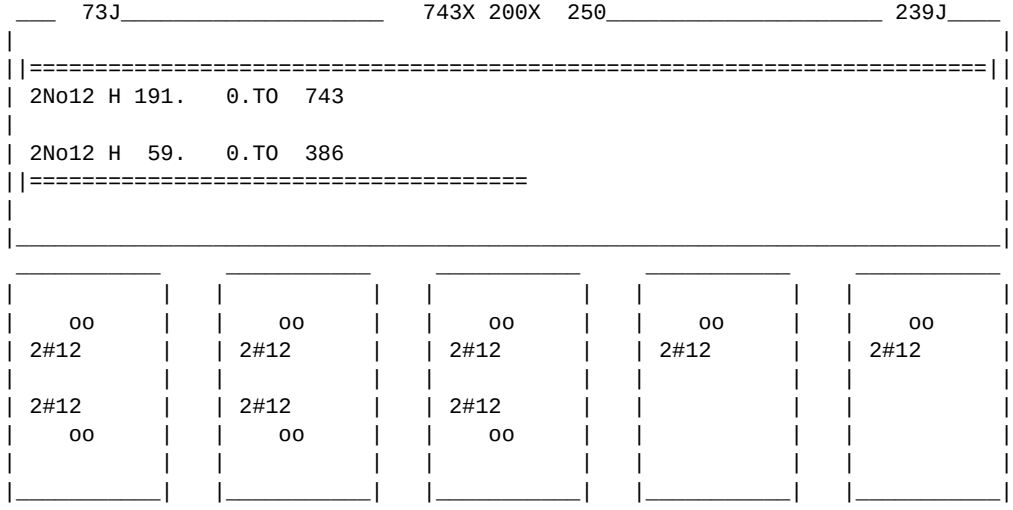
BEAM NO. 361 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 743. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	386.	YES	NO
2	191.	2 - 12MM	0.	743.	YES	YES

B E A M N O. 361 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 11.32 KNS Tu= 6.2 KN-MET
Vc= 36.6 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 6 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 11.82 KNS Tu= 6.2 KN-MET
Vc= 28.1 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 6 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



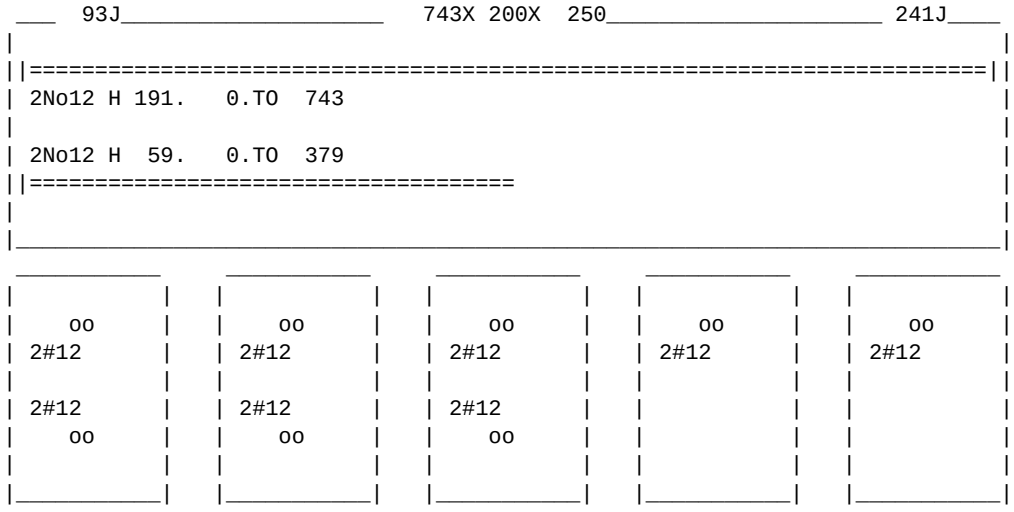
BEAM NO. 362 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 743. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	379.	YES	NO
2	191.	2 - 12MM	0.	743.	YES	YES

B E A M N O. 362 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 11.58 KNS Tu= 5.3 KN-MET
Vc= 37.3 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 7 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 12.08 KNS Tu= 5.3 KN-MET
Vc= 28.1 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 7 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



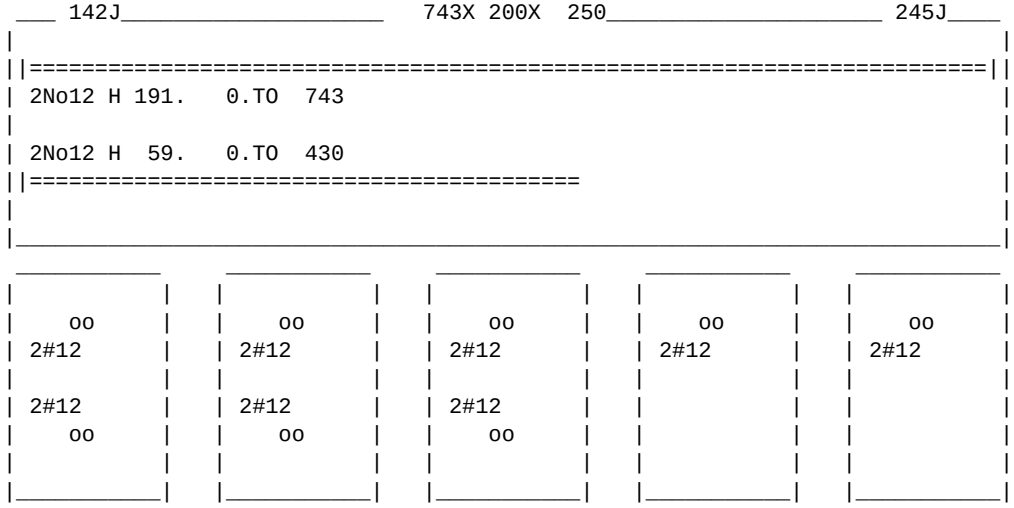
BEAM NO. 363 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 743. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	430.	YES	NO
2	191.	2 - 12MM	0.	743.	YES	YES

B E A M N O. 363 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 18.11 KNS Tu= 3.7 KN-MET
Vc= 51.7 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 6 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 18.61 KNS Tu= 3.7 KN-MET
Vc= 28.4 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 6 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



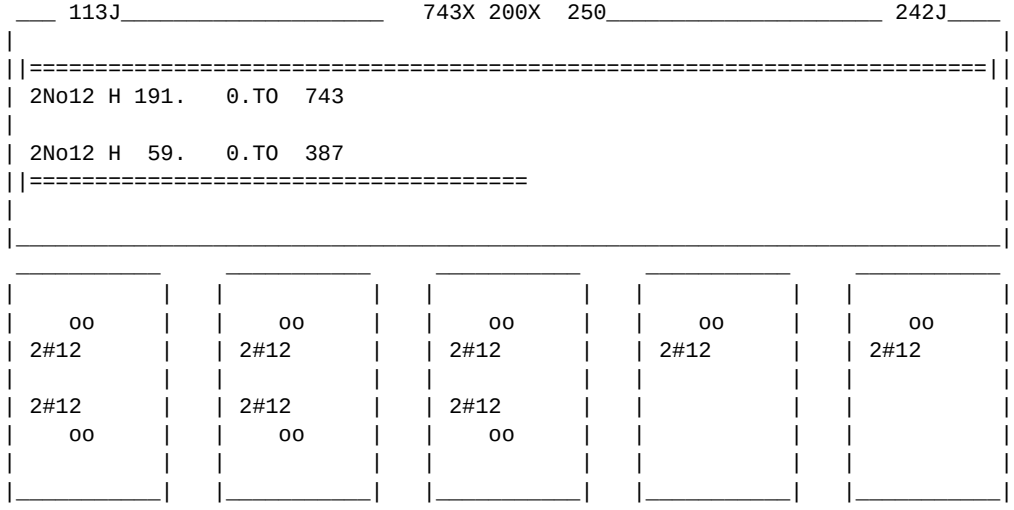
BEAM NO. 364 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 743. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	387.	YES	NO
2	191.	2 - 12MM	0.	743.	YES	YES

B E A M N O. 364 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 11.29 KNS Tu= 5.9 KN-MET
Vc= 37.0 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 6 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 11.78 KNS Tu= 5.9 KN-MET
Vc= 28.1 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 6 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



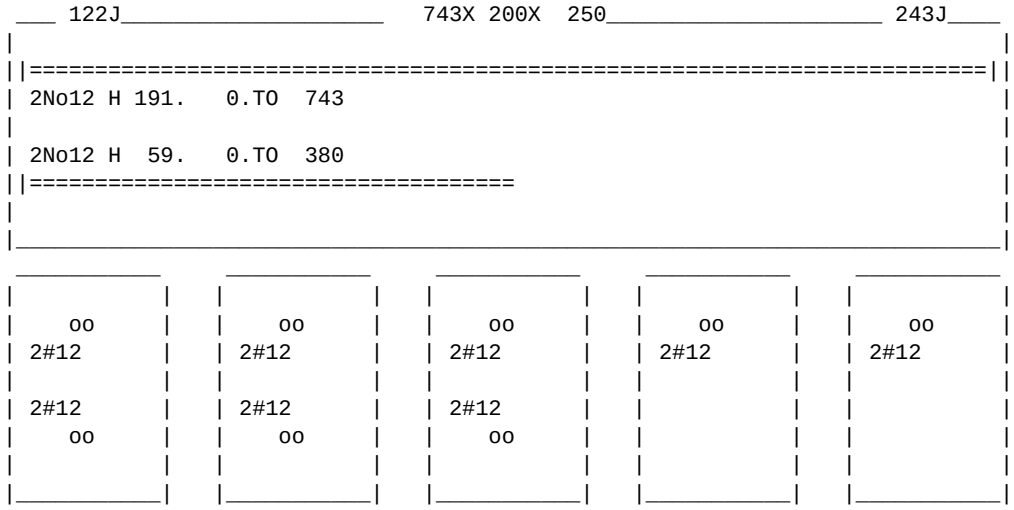
BEAM NO. 365 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 743. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	380.	YES	NO
2	191.	2 - 12MM	0.	743.	YES	YES

B E A M N O. 365 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 11.60 KNS Tu= 5.7 KN-MET
Vc= 37.3 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 7 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 14.02 KNS Tu= 2.6 KN-MET
Vc= 28.1 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



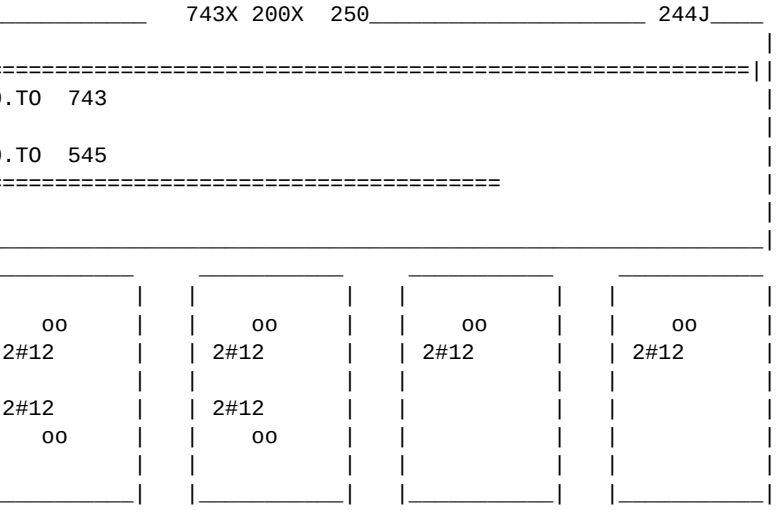
BEAM NO. 366 DESIGN RESULTS - FLEXURE PER CODE ACI 318-08

LEN - 743. MM FY - 420. FC - 21. MPA, SIZE - 200. X 250. MMS

LEVEL	HEIGHT (MM)	BAR INFO	FROM (MM)	TO (MM)	ANCHOR STA	ANCHOR END
1	59.	2 - 12MM	0.	545.	YES	NO
2	191.	2 - 12MM	0.	743.	YES	YES

B E A M N O. 366 D E S I G N R E S U L T S - SHEAR

AT START SUPPORT - Vu= 8.38 KNS Tu= 4.3 KN-MET
Vc= 51.9 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.
AT END SUPPORT - Vu= 8.87 KNS Tu= 4.3 KN-MET
Vc= 28.0 KNS, ACI 318:CLAUSE 11.6.3.1
LOAD 5 TORSION VALUE TOO HIGH, INCREASE MEMBER SIZE.



**********END OF BEAM DESIGN**********

399. DESIGN COLUMN 12 TO 15 17 TO 21 23 TO 26 28 TO 30 32 TO 42 44 47 48 50 53 -
400. 54 56 65 66 68 71 72 74 77 78 80 114 118 122 130 134 138 152 158 161 164 -
401. 168 174 TO 176 353 TO 359 367 TO 395 430 431 452 TO 457

==========

COLUMN NO. 12 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 13 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 14 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 15 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 17 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 18 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 19 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 20 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 21 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 23 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 24 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 25 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 26 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 28 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 29 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 30 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 32 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
AREA OF STEEL REQUIRED = 800.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	13	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 33 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
AREA OF STEEL REQUIRED = 800.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	11	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 34 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
AREA OF STEEL REQUIRED = 800.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	13	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)

TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 35 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
AREA OF STEEL REQUIRED = 800.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	11	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 36 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
AREA OF STEEL REQUIRED = 800.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	13	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 37 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
AREA OF STEEL REQUIRED = 800.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI

4 - 16 MM	1.287	11	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 38 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
AREA OF STEEL REQUIRED = 800.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	13	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 39 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
AREA OF STEEL REQUIRED = 843.7 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
8 - 12 MM	1.448	11	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 192.00 MM

==========

COLUMN NO. 40 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 41 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 42 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 44 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 47 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 48 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 50 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 53 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 54 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 56 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 65 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 66 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 68 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 71 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 72 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 74 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 77 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 78 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 80 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 114 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 118 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 122 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 130 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 134 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 138 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 152 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 158 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 161 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 164 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 168 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 174 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 175 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 176 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 353 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 354 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 355 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 356 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 357 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 358 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 359 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 367 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 20 MM 1.026 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 368 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 20 MM 1.026 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 369 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 20 MM 1.026 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 370 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 371 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 372 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 373 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 374 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 375 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 20 MM 1.026 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 376 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 20 MM 1.026 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 377 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 20 MM 1.026 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 378 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 379 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 380 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 381 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 382 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 383 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 20 MM 1.026 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 384 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 20 MM 1.026 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 385 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 20 MM 1.026 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 386 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 387 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 388 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 389 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 390 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 391 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 20 MM 1.026 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 392 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 20 MM 1.026 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 393 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 20 MM 1.026 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 394 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 395 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 350.0 X 350.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 1225.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 20 MM	1.026	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 320.00 MM

==========

COLUMN NO. 430 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
AREA OF STEEL REQUIRED = 756.2 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	10	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 431 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 452 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
AREA OF STEEL REQUIRED = 712.5 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	10	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 453 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 454 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 455 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
AREA OF STEEL REQUIRED = 712.5 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	10	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 456 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
AREA OF STEEL REQUIRED = 756.2 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 10 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

==========

COLUMN NO. 457 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

**********END OF COLUMN DESIGN RESULTS**********

402. END CONCRETE DESIGN
403. PARAMETER 2
404. CODE LRFD
405. AXIS 1 MEMB 143 146 148 151 206 208 213 214 218 221 224 227 230 233 236 239 -
406. 440 TO 451 466 TO 477 486 TO 497 506 TO 581
407. FU 500000 MEMB 143 146 148 151 206 208 213 214 218 221 224 227 230 233 236 -
408. 239 440 TO 451 466 TO 477 486 TO 497 506 TO 581
409. FYLD 350000 MEMB 143 146 148 151 206 208 213 214 218 221 224 227 230 233 236 -
410. 239 440 TO 451 466 TO 477 486 TO 497 506 TO 581
411. SELECT MEMB 143 146 148 151 206 208 213 214 218 221 224 227 230 233 236 239 -
412. 440 TO 451 466 TO 477 486 TO 497 506 TO 581

STAAD.Pro MEMBER SELECTION - (LRFD 3RD EDITION) v1.0
****

ALL UNITS ARE - KN METE (UNLESS OTHERWISE Noted)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 143). PLEASE DEFINE THE PROPERTIES THROUGH A USER TABLE GENERAL SECTION.					
**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 146). PLEASE DEFINE THE PROPERTIES THROUGH A USER TABLE GENERAL SECTION.					
148 ST	UPN120		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.215	6
		1.07 C	-0.15	2.77	0.00
151 ST	UPN120		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.215	9
		1.23 C	-0.15	2.75	3.12
206 ST	UPN120		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.339	8
		0.56 C	0.16	4.74	3.12
208 ST	UPN120		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.121	9
		0.32 C	0.10	1.53	0.00
213 ST	UPN120		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.335	7
		0.56 C	0.16	4.68	0.00
214 ST	UPN120		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.121	9
		0.29 C	0.10	1.55	0.00
218 ST	UPN120		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.341	9
		0.01 C	-0.19	4.76	3.12
221 ST	UPN120		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-T	0.342	6
		0.08 T	-0.18	4.79	0.00
224 ST	UPN120		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.119	13
		0.03 C	0.20	1.26	0.00
227 ST	UPN120		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-T	0.119	13
		0.14 T	0.20	1.26	0.00
230 ST	UPN80		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-T	0.619	8
		2.59 T	0.13	4.06	0.00
233 ST	UPN80		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-T	0.622	8
		2.49 T	0.12	4.11	0.00

ALL UNITS ARE - KN METE (UNLESS OTHERWISE Noted)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
236	ST UPN120		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-T	0.317	6
		0.19 T	-0.17	4.44	3.12
239	ST UPN120		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-T	0.318	6
		0.16 T	-0.17	4.44	3.12

**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 440).
PLEASE DEFINE THE PROPERTIES THROUGH A USER TABLE GENERAL SECTION.

**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 441).
PLEASE DEFINE THE PROPERTIES THROUGH A USER TABLE GENERAL SECTION.

**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 442).
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**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 443).
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**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 447).
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**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 448).
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**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 469).
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**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 475).
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**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 517).
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**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 518).
PLEASE DEFINE THE PROPERTIES THROUGH A USER TABLE GENERAL SECTION.

**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 519).
PLEASE DEFINE THE PROPERTIES THROUGH A USER TABLE GENERAL SECTION.

520 ST UPN180 (EUROPEAN SECTIONS)
PASS LRFD-H1-1B-C 0.206 12
0.18 C -0.33 5.47 3.95

ALL UNITS ARE - KN METE (UNLESS OTHERWISE Noted)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
521 ST	UPN200	(EUROPEAN SECTIONS)			
		PASS	LRFD-H1-1B-C	0.156	9
		0.03 C	0.27	5.37	4.05
522 ST	UPN180	(EUROPEAN SECTIONS)			
		PASS	LRFD-H1-1B-C	0.249	13
		8.43 C	-0.28	5.77	3.95
523 ST	UPN200	(EUROPEAN SECTIONS)			
		PASS	LRFD-H1-1B-C	0.177	5
		3.46 C	-0.11	6.21	0.00
524 ST	UPN180	(EUROPEAN SECTIONS)			
		PASS	LRFD-H1-1B-T	0.095	12
		0.93 T	-0.36	1.92	3.95
525 ST	UPN200	(EUROPEAN SECTIONS)			
		PASS	LRFD-H1-1B-T	0.079	11
		0.04 T	-0.28	2.27	0.00

**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 526).
PLEASE DEFINE THE PROPERTIES THROUGH A USER TABLE GENERAL SECTION.

527 ST	UPN180	(EUROPEAN SECTIONS)			
		PASS	LRFD-H1-1B-T	0.198	12
		0.42 T	-0.31	5.24	4.00
528 ST	UPN180	(EUROPEAN SECTIONS)			
		PASS	LRFD-H1-1B-C	0.211	12
		1.21 C	-0.32	5.45	4.00
529 ST	UPN180	(EUROPEAN SECTIONS)			
		PASS	LRFD-H1-1B-T	0.091	12
		0.73 T	-0.35	1.79	4.00

**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 530).
PLEASE DEFINE THE PROPERTIES THROUGH A USER TABLE GENERAL SECTION.

531 ST	UPN180	(EUROPEAN SECTIONS)			
		PASS	LRFD-H1-1B-T	0.211	5
		0.32 T	0.09	6.29	4.00
532 ST	UPN180	(EUROPEAN SECTIONS)			
		PASS	LRFD-H1-1B-C	0.205	5
		0.54 C	-0.11	5.95	0.00
533 ST	UPN180	(EUROPEAN SECTIONS)			
		PASS	LRFD-H1-1B-C	0.096	11
		0.28 C	-0.28	2.12	0.00
534 ST	UPN180	(EUROPEAN SECTIONS)			
		PASS	LRFD-H1-1B-T	0.089	12
		0.61 T	0.33	1.80	0.00

ALL UNITS ARE - KN METE (UNLESS OTHERWISE Noted)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
535 ST	UPN180	(EUROPEAN SECTIONS)			
	PASS	LRFD-H1-1B-C	0.203		12
	0.23 C	-0.31	5.44		3.95
536 ST	UPN180	(EUROPEAN SECTIONS)			
	PASS	LRFD-H1-1B-T	0.205		6
	0.34 T	0.22	5.80		0.00

**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 537).
PLEASE DEFINE THE PROPERTIES THROUGH A USER TABLE GENERAL SECTION.

**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 538).
PLEASE DEFINE THE PROPERTIES THROUGH A USER TABLE GENERAL SECTION.

539 ST	UPN200	(EUROPEAN SECTIONS)			
	PASS	LRFD-H1-1B-T	0.157		9
	0.28 T	0.27	5.41		4.05
540 ST	UPN200	(EUROPEAN SECTIONS)			
	PASS	LRFD-H1-1B-C	0.162		5
	1.20 C	-0.10	5.96		0.00
541 ST	UPN200	(EUROPEAN SECTIONS)			
	PASS	LRFD-H1-1B-C	0.074		11
	0.05 C	-0.26	2.12		0.00
542 ST	UPN200	(EUROPEAN SECTIONS)			
	PASS	LRFD-H1-1B-T	0.074		12
	0.77 T	-0.34	1.89		4.05
543 ST	UPN200	(EUROPEAN SECTIONS)			
	PASS	LRFD-H1-1B-C	0.175		5
	3.36 C	-0.11	6.14		4.05
544 ST	UPN200	(EUROPEAN SECTIONS)			
	PASS	LRFD-H1-1B-T	0.158		12
	0.80 T	-0.28	5.42		4.05

**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 545).
PLEASE DEFINE THE PROPERTIES THROUGH A USER TABLE GENERAL SECTION.

546 ST	UPN80	(EUROPEAN SECTIONS)			
	PASS	LRFD-H1-1B-T	0.460		11
	0.41 T	-0.24	2.27		0.00
547 ST	UPN180	(EUROPEAN SECTIONS)			
	PASS	LRFD-H1-1B-C	0.240		5
	7.73 C	-0.14	6.14		0.00
548 ST	UPN180	(EUROPEAN SECTIONS)			
	PASS	LRFD-H1-1B-C	0.190		5
	0.30 C	-0.14	5.62		0.00

**WARNING-THIS VERSION DOES NOT DESIGN FACE-TO-FACE CHANNEL SECTIONS (MEMBER 549).
PLEASE DEFINE THE PROPERTIES THROUGH A USER TABLE GENERAL SECTION.

550	ST	UPN180	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-C	0.152	9		
		5.84 C	0.20	3.41	0.00		
551	ST	UPN200	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-C	0.141	6		
		7.29 C	0.22	3.85	4.05		
552	ST	UPN200	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-C	0.126	9		
		5.81 C	0.18	3.60	0.00		
553	ST	UPN180	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-C	0.208	9		
		4.36 C	0.23	5.31	0.00		
554	ST	UPN180	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-C	0.195	5		
		0.90 C	-0.13	5.70	3.90		
555	ST	UPN180	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-C	0.091	13		
		1.28 C	-0.19	2.16	3.90		
556	ST	UPN200	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-T	0.068	10		
		0.64 T	-0.27	1.88	0.00		
557	ST	UPN200	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-C	0.170	10		
		0.75 C	-0.34	5.62	0.00		
558	ST	UPN200	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-C	0.195	5		
		3.81 C	0.05	7.07	4.05		
559	ST	UPN200	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-C	0.188	5		
		2.76 C	0.03	7.03	0.00		
560	ST	UPN200	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-C	0.164	7		
		0.85 C	0.25	5.61	0.00		
561	ST	UPN200	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-C	0.067	13		
		0.74 C	0.20	1.96	0.00		
562	ST	UPN180	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-C	0.171	6		
		4.48 C	0.15	4.28	3.95		
563	ST	UPN180	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-C	0.255	5		
		2.52 C	0.02	7.62	3.95		
564	ST	UPN180	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-C	0.214	8		
		0.31 C	-0.21	6.06	0.00		
565	ST	UPN180	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-T	0.079	10		
		0.49 T	-0.26	1.73	0.00		
566	ST	UPN180	(EUROPEAN SECTIONS)				
		PASS	LRFD-H1-1B-C	0.173	9		
		4.67 C	0.14	4.26	0.00		

ALL UNITS ARE - KN METE (UNLESS OTHERWISE Noted)

MEMBER	TABLE	RESULT/ FX	CRITICAL COND/ MY	RATIO/ MZ	LOADING/ LOCATION
567 ST	UPN180		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.263	5
		2.57 C	0.02	7.72	0.00
568 ST	UPN180		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.219	7
		0.44 C	0.22	6.09	0.00
569 ST	UPN180		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.088	13
		0.81 C	-0.22	1.96	4.00
570 ST	UPN180		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.162	6
		5.69 C	0.20	3.59	4.00
571 ST	UPN180		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.237	5
		2.75 C	0.03	6.87	4.00
572 ST	UPN180		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.206	10
		0.28 C	-0.33	5.38	0.00
573 ST	UPN180		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-T	0.081	10
		0.21 T	-0.27	1.73	0.00
574 ST	UPN200		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.141	9
		7.51 C	0.21	3.85	0.00
575 ST	UPN200		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.195	5
		3.85 C	0.05	7.06	0.00
576 ST	UPN200		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.165	7
		1.17 C	0.29	5.54	0.00
577 ST	UPN200		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.073	13
		0.56 C	-0.22	2.15	4.05
578 ST	UPN180		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.158	6
		5.83 C	0.23	3.44	3.95
579 ST	UPN180		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.215	6
		4.08 C	0.27	5.36	3.95
580 ST	UPN180		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.216	10
		0.58 C	-0.36	5.66	0.00
581 ST	UPN180		(EUROPEAN SECTIONS)		
		PASS	LRFD-H1-1B-C	0.090	10
		0.56 C	-0.29	1.91	0.00

********** END OF TABULATED RESULT OF DESIGN **********

413. FINISH

****
WARNING SOME MEMBER SIZES HAVE CHANGED SINCE LAST ANALYSIS.
IN THE POST PROCESSOR, MEMBER QUERIES WILL USE THE LAST
ANALYSIS FORCES WITH THE UPDATED MEMBER SIZES.
TO CORRECT THIS INCONSISTENCY, PLEASE DO ONE MORE ANALYSIS.
FROM THE UPPER MENU, PRESS RESULTS, UPDATE PROPERTIES, THEN
FILE SAVE; THEN ANALYZE AGAIN WITHOUT THE GROUP OR SELECT
COMMANDS.
****

********** END OF THE STAAD.Pro RUN **********

**** DATE= DEC 22,2014 TIME= 10: 2:18 ****

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*************************************************************
*        For questions on STAAD.Pro, please contact         *
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