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*
*          STAAD.Pro V8i SELECTseries5          *
*          Version  20.07.10.64                *
*          Proprietary Program of              *
*          Bentley Systems, Inc.                *
*          Date=    DEC 22, 2014                *
*          Time=    18:47:41                    *
*
*          USER ID:                             *
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1. STAAD SPACE DXF IMPORT OF MODELO 3D FALTANTE.DXF
INPUT FILE: Comedor Servicios Gen.STD
2. START JOB INFORMATION
3. ENGINEER DATE 22-DEC-14
4. JOB NAME STAAD COMEDOR Y SERVICIOS GENERALES
5. ENGINEER NAME EMEL MULET
6. END JOB INFORMATION
7. INPUT WIDTH 79
8. UNIT METER KN
9. JOINT COORDINATES
10. 17 21.9521 1 -4.8772; 19 26.1921 1 -4.8772; 20 26.1921 3.7 -4.8772
11. 21 32.0085 1 -4.8772; 22 32.0085 3.7 -4.8772; 23 32.0085 1 8.92053
12. 24 32.0085 3.7 8.92053; 25 35.7185 1 8.92053; 26 35.7185 3.7 8.92053
13. 27 39.4685 1 8.92053; 28 39.4685 3.7 8.92053; 29 43.2185 1 8.92053
14. 30 43.2185 3.7 8.92053; 31 46.9746 1 8.92053; 32 46.9746 3.7 8.92053
15. 33 45.7686 1 0.000366211; 34 45.7686 4.9 0.000366211; 35 43.2185 1 0.000366211
16. 36 43.2185 4.9 0.000366211; 37 39.4685 1 0.000366211
17. 38 39.4685 4.9 0.000366211; 39 35.7185 1 0.000366211
18. 40 35.7185 4.9 0.000366211; 41 46.9685 1 17.7612; 42 46.9685 4.9 17.7612
19. 43 43.2185 1 17.7612; 44 43.2185 4.9 17.7612; 45 39.4685 1 17.7612
20. 46 39.4685 4.9 17.7612; 47 35.7185 1 17.7612; 48 35.7185 4.9 17.7612
21. 49 32.0085 1 17.7612; 50 32.0085 4.9 17.7612; 51 26.1921 3.58865 -5.83215
22. 52 26.1921 4.5 1.98352; 53 32.0085 3.58865 -5.83215; 54 32.0085 4.5 1.98352
23. 55 26.1921 4.05855 5.8114; 56 26.1921 1 4.67883; 57 26.1921 4.18917 4.67883
24. 58 32.0085 1 4.67883; 59 32.0085 4.18917 4.67883; 60 39.4685 5.00426 -0.774658
25. 61 43.2185 5.00426 -0.774658; 62 32.0085 1 0.000366211
26. 63 32.0085 4.26875 0.000366211; 64 26.1921 1 0.000366211
27. 65 26.1921 4.26875 0.000366211; 66 21.9521 1 0.00012207
28. 68 21.9521 3.58865 -5.83215; 69 21.9521 4.32627 0.493652
29. 70 35.7185 4.26875 0.000366211; 71 35.7185 3.80507 -3.97607
30. 72 35.7185 4.5 1.98352; 73 32.0085 1 13.3408; 74 32.0085 4.3 13.3408
31. 75 35.7185 1 13.3408; 76 35.7185 4.3 13.3408; 77 39.4685 1 13.3408
32. 78 39.4685 4.3 13.3408; 79 43.2185 1 13.3408; 80 43.2185 4.3 13.3408
33. 81 46.9685 1 13.3408; 82 46.9685 4.3 13.3408; 83 46.9714 4.3 13.3408
34. 84 39.4685 1 4.67883; 85 39.4685 4.27062 4.67883; 86 43.2185 1 4.67883
35. 87 43.2185 4.27062 4.67883; 88 45.7686 1 4.67883; 89 45.7686 4.27062 4.67883
36. 90 35.7185 1 4.67883; 91 35.7185 4.27062 4.67883; 92 45.7686 5.00426 -0.774658
37. 93 45.7686 3.7 8.92053; 94 21.9521 4.26875 0.000366211; 95 21.9521 3.7 -4.8772
38. 105 35.7185 4.18917 4.67883; 106 24.572 4.5 1.98352
    
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39. 111 21.9521 3.7 0.00012207; 112 30.0686 1 8.92053; 113 30.0686 3.7 8.92053
40. 114 30.0686 4.18917 4.67883; 115 30.0686 4.05855 5.8114
41. 116 46.9746 4.27062 4.67883; 117 35.7185 5.00426 -0.774658
42. 122 24.572 4.26875 0.000366211; 125 24.572 4.18917 4.67883
43. 126 32.0085 4.69825 16.2748; 127 46.9695 4.69961 16.2849
44. 128 32.0085 4.49649 14.7885; 130 32.0085 4.09825 11.8545
45. 134 35.7185 4.80427 0.711914; 135 45.7686 4.80427 0.711914
46. 136 35.7185 4.60428 2.19861; 137 45.7686 4.60428 2.19861
47. 138 35.7185 4.40429 3.68518; 139 45.7686 4.40429 3.68518
48. 140 35.7185 4.2043 5.17175; 141 46.9746 4.2043 5.17175
49. 142 35.7185 4.00431 6.65845; 143 46.9746 4.00431 6.65845
50. 144 35.7185 3.80432 8.14502; 145 46.9746 3.80432 8.14502
51. 156 32.0085 4.32815 3.47363; 157 24.572 4.32815 3.47363
52. 158 35.7185 4.32815 3.47363; 159 26.1921 4.1563 4.96375
53. 160 35.7185 4.1563 4.96375; 161 30.0686 3.81261 7.94409
54. 162 35.7185 3.81261 7.94409; 163 30.0686 3.98446 6.45386
55. 164 35.7185 3.98446 6.45386; 165 35.7185 4.32627 0.493652
56. 166 21.9521 4.15254 -0.996216; 167 35.7185 4.15254 -0.996216
57. 168 21.9521 3.97881 -2.48621; 169 35.7185 3.97881 -2.48621
58. 170 21.9521 3.80507 -3.97607; 171 21.9521 3.63134 -5.46594
59. 172 32.0085 3.63134 -5.46594; 181 21.9521 0 -4.8772; 182 26.1921 0 -4.8772
60. 183 32.0085 0 -4.8772; 184 32.0085 0 8.92053; 185 35.7185 0 8.92053
61. 186 39.4685 0 8.92053; 187 43.2185 0 8.92053; 188 46.9746 0 8.92053
62. 189 45.7686 0 0.000366211; 190 43.2185 0 0.000366211
63. 191 39.4685 0 0.000366211; 192 35.7185 0 0.000366211; 193 46.9685 0 17.7612
64. 194 43.2185 0 17.7612; 195 39.4685 0 17.7612; 196 35.7185 0 17.7612
65. 197 32.0085 0 17.7612; 198 26.1921 0 4.67883; 199 32.0085 0 4.67883
66. 200 32.0085 0 0.000366211; 201 26.1921 0 0.000366211; 202 21.9521 0 0.00012207
67. 203 32.0085 0 13.3408; 204 35.7185 0 13.3408; 205 39.4685 0 13.3408
68. 206 43.2185 0 13.3408; 207 46.9685 0 13.3408; 208 39.4685 0 4.67883
69. 209 43.2185 0 4.67883; 210 45.7686 0 4.67883; 211 35.7185 0 4.67883
70. 213 30.0686 0 8.92053; 214 26.1921 4.32627 0.493652
71. 215 26.1921 4.15255 -0.996216; 216 26.1921 3.97881 -2.48621
72. 217 26.1921 3.80508 -3.97607; 218 26.1921 3.63135 -5.46594
73. 219 32.0085 4.32627 0.493652; 220 32.0085 4.15255 -0.996216
74. 221 32.0085 3.97881 -2.48621; 222 32.0085 3.80508 -3.97607
75. 223 32.0085 3.81261 7.94409; 224 32.0085 3.98446 6.45386
76. 225 32.0085 4.15631 4.96375; 226 26.1921 4.32815 3.47363
77. 227 39.4685 3.80433 8.14502; 228 39.4685 4.00431 6.65845
78. 229 39.4685 4.20431 5.17175; 230 39.4685 4.40429 3.68518
79. 231 39.4685 4.60428 2.19861; 232 39.4685 4.80428 0.711915
80. 233 43.2185 3.80433 8.14502; 234 43.2185 4.00431 6.65845
81. 235 43.2185 4.20431 5.17175; 236 43.2185 4.40429 3.68518
82. 237 43.2185 4.60428 2.19861; 238 43.2185 4.80428 0.711915
83. 239 21.9521 4.03241 -2.02647; 240 43.2185 4.69927 16.2824
84. 241 43.2185 4.49751 14.796; 242 43.2185 4.09926 11.862
85. 243 43.2185 3.89751 10.3756; 244 39.4685 4.69893 16.2798
86. 245 39.4685 4.49717 14.7935; 246 39.4685 4.09892 11.8595
87. 247 39.4685 3.89716 10.3731; 248 35.7185 4.69858 16.2773
88. 249 35.7185 4.49683 14.791; 250 35.7185 4.09858 11.857
89. 251 35.7185 3.89682 10.3705; 252 45.7686 3.80433 8.14502
90. 253 45.7686 4.00431 6.65845; 254 45.7686 4.20431 5.17175
91. 273 30.0686 4.15631 4.96375; 274 24.572 4.32627 0.493652
92. 276 46.9735 3.9 10.394; 277 46.9725 4.1 11.8674; 278 32.0085 3.89913 10.3875
93. 279 46.9705 4.49784 14.7984; 280 35.7185 1 -3.13963; 282 35.7185 0 -3.13963
94. 283 35.7185 3.90261 -3.13963; 284 32.0085 1 -3.13963

95. MEMBER INCIDENCES

96. 9 17 95; 10 19 20; 11 21 22; 12 23 24; 13 25 26; 14 27 28; 15 29 30; 16 31 32
97. 17 33 34; 18 35 36; 19 37 38; 20 39 70; 21 41 42; 22 43 44; 23 45 46; 24 47 48
98. 25 49 50; 26 51 218; 27 53 172; 28 54 156; 29 52 226; 30 56 57; 31 58 59
99. 32 40 134; 33 60 38; 34 61 36; 35 32 93; 36 30 28; 37 28 26; 38 26 24
100. 39 40 38; 40 38 36; 41 36 34; 42 62 63; 43 64 65; 44 66 111; 45 68 171
101. 46 63 70; 47 71 283; 48 72 158; 49 32 276; 50 30 243; 51 28 247; 52 26 251
102. 53 24 278; 54 73 74; 55 75 76; 56 77 78; 57 79 80; 58 81 82; 59 83 82
103. 60 80 78; 61 78 76; 62 76 74; 63 50 48; 64 48 46; 65 46 44; 66 44 42; 67 84 85
104. 68 86 87; 69 88 89; 70 90 105; 71 92 34; 72 89 87; 73 87 85; 74 85 91
105. 75 63 65; 76 65 122; 77 95 20; 78 20 22; 90 57 114; 91 59 105; 92 72 54
106. 93 54 52; 94 52 106; 101 112 113; 102 24 113; 103 113 161; 104 55 115
107. 105 116 141; 106 89 116; 107 117 60; 108 117 40; 109 60 61; 110 61 92
108. 118 122 274; 121 57 125; 122 106 157; 123 126 248; 124 128 249; 125 130 250
109. 127 134 232; 128 136 231; 129 138 230; 130 140 229; 131 142 228; 132 144 227
110. 139 157 226; 140 159 273; 141 161 223; 142 163 224; 143 69 274; 144 166 215
111. 145 168 216; 146 170 217; 147 171 218; 156 17 181; 157 19 182; 158 21 183
112. 159 23 184; 160 25 185; 161 27 186; 162 29 187; 163 31 188; 164 33 189
113. 165 35 190; 166 37 191; 167 39 192; 168 41 193; 169 43 194; 170 45 195
114. 171 47 196; 172 49 197; 173 56 198; 174 58 199; 175 62 200; 176 64 201
115. 177 66 202; 178 73 203; 179 75 204; 180 77 205; 181 79 206; 182 81 207
116. 183 84 208; 184 86 209; 185 88 210; 186 90 211; 188 112 213; 193 17 19
117. 194 19 21; 195 21 284; 196 62 58; 197 58 23; 198 23 73; 199 73 49; 200 62 39
118. 201 39 37; 202 37 35; 203 35 33; 204 56 58; 205 58 90; 206 90 84; 207 84 86
119. 208 86 88; 209 112 23; 210 23 25; 211 25 27; 212 27 29; 213 29 31; 214 73 75
120. 215 75 77; 216 77 79; 217 79 81; 218 49 47; 219 47 45; 220 45 43; 221 43 41
121. 222 33 88; 223 88 31; 224 31 81; 225 81 41; 226 35 86; 227 86 29; 228 29 79
122. 229 79 43; 230 37 84; 231 84 27; 232 27 77; 233 77 45; 234 39 90; 235 90 25
123. 236 25 75; 237 75 47; 238 19 64; 239 64 56; 240 62 64; 241 64 66; 252 17 66
124. 253 56 112; 255 70 40; 256 214 52; 257 65 214; 258 215 65; 259 216 215
125. 260 217 216; 261 20 217; 262 218 20; 263 219 54; 264 63 219; 265 220 63
126. 266 221 220; 267 222 221; 268 22 222; 269 172 22; 270 223 24; 271 224 223
127. 272 225 224; 273 59 225; 274 156 59; 275 159 55; 276 57 159; 277 226 57
128. 278 144 26; 279 142 144; 280 140 142; 281 91 140; 282 138 91; 283 136 138
129. 284 134 136; 285 227 28; 286 228 227; 287 229 228; 288 85 229; 289 230 85
130. 290 231 230; 291 232 231; 292 38 232; 293 233 30; 294 234 233; 295 235 234
131. 296 87 235; 297 236 87; 298 237 236; 299 238 237; 300 36 238; 301 93 30
132. 302 111 94; 303 94 69; 304 166 94; 305 239 166; 306 168 239; 307 170 168
133. 308 95 170; 309 171 95; 310 165 72; 311 70 165; 312 167 70; 313 169 167
134. 314 162 26; 315 164 162; 316 160 164; 317 105 160; 318 158 105; 319 127 42
135. 320 83 279; 321 240 44; 322 241 240; 323 80 241; 324 242 80; 325 243 242
136. 326 244 46; 327 245 244; 328 78 245; 329 246 78; 330 247 246; 331 248 48
137. 332 249 248; 333 76 249; 334 250 76; 335 251 250; 336 126 50; 337 128 126
138. 338 74 128; 339 130 74; 340 82 80; 341 105 91; 342 252 93; 343 253 252
139. 344 254 253; 345 89 254; 346 139 89; 347 137 139; 348 135 137; 349 34 135
140. 350 122 94; 385 114 59; 392 273 114; 393 115 273; 394 163 115; 395 161 163
141. 396 145 32; 397 143 145; 398 141 143; 399 274 106; 401 157 125; 402 240 127
142. 403 244 240; 404 248 244; 406 245 241; 407 249 245; 409 246 242; 410 250 246
143. 412 247 243; 413 251 247; 414 238 135; 415 232 238; 416 237 137; 417 231 237
144. 418 236 139; 419 230 236; 420 254 141; 421 235 254; 422 229 235; 423 253 143
145. 424 234 253; 425 228 234; 426 252 145; 427 233 252; 428 227 233; 448 156 158
146. 449 226 156; 450 225 160; 451 273 225; 452 223 162; 453 224 164; 454 219 165
147. 455 214 219; 456 274 214; 457 220 167; 458 215 220; 459 221 169; 460 216 221
148. 461 222 71; 462 217 222; 463 218 172; 464 276 277; 465 277 83; 466 243 276
149. 467 242 277; 468 278 130; 469 251 278; 470 279 127; 471 241 279; 472 280 283
150. 473 280 282; 474 283 169; 475 280 39; 476 284 62; 477 280 284

151. DEFINE MATERIAL START
152. ISOTROPIC CONCRETE
153. E 1.7872E+007
154. POISSON 0.17
155. DENSITY 23.5616
156. ALPHA 1E-005
157. DAMP 0.05
158. TYPE CONCRETE
159. STRENGTH FCU 21000
160. ISOTROPIC STEEL
161. E 2.05E+008
162. POISSON 0.3
163. DENSITY 76.8195
164. ALPHA 1.2E-005
165. DAMP 0.03
166. TYPE STEEL
167. STRENGTH FY 253200 FU 407800 RY 1.5 RT 1.2
168. END DEFINE MATERIAL
169. MEMBER PROPERTY AMERICAN
170. 9 TO 25 30 31 42 TO 44 54 TO 58 67 TO 70 101 156 TO 186 188 255 302 341 472 -
171. 473 PRIS YD 0.25 ZD 0.25
172. MEMBER PROPERTY COLDFORMED AMERICAN
173. 26 TO 29 32 TO 41 45 TO 53 59 TO 66 71 TO 78 90 91 102 103 105 106 108 121 -
174. 256 TO 301 303 TO 340 342 TO 350 385 392 TO 398 464 465 468 470 -
175. 474 TABLE ST 14CS3.75X075
176. 92 TO 94 104 107 109 110 118 122 TO 125 127 TO 132 139 TO 147 399 401 TO 404 -
177. 406 407 409 410 412 TO 428 448 TO 463 466 467 469 -
178. 471 TABLE ST 14CS3.75X075
179. MEMBER PROPERTY COLDFORMED AMERICAN
180. 193 TO 241 252 253 475 TO 477 PRIS YD 0.35 ZD 0.35
181. CONSTANTS
182. MATERIAL CONCRETE MEMB 9 TO 25 30 31 42 TO 44 54 TO 58 67 TO 70 101 -
183. 156 TO 186 188 193 TO 241 252 253 255 302 341 472 473 475 TO 477
184. MATERIAL STEEL MEMB 26 TO 29 32 TO 41 45 TO 53 59 TO 66 71 TO 78 90 TO 94 -
185. 102 TO 110 118 121 TO 125 127 TO 132 139 TO 147 256 TO 301 303 TO 340 342 -
186. 343 TO 350 385 392 TO 399 401 TO 404 406 407 409 410 412 TO 428 448 TO 471 -
187. 474
188. SUPPORTS
189. 181 TO 211 213 282 FIXED
190. DEFINE COLOMBIAN ACCIDENTAL LOAD
191. ZONE 0.2 I 1 S 1.5
192. SELFWEIGHT 1
193. MEMBER WEIGHT
194. 92 TO 94 104 107 109 110 123 TO 125 127 TO 132 139 TO 147 402 TO 404 406 407 -
195. 409 410 412 TO 428 448 TO 463 466 467 469 471 UNI 1.2
196. LOAD 1 SISMO X
197. COLOMBIAN LOAD X 1 ACC 1
198. LOAD 2 SISMO Z
199. COLOMBIAN LOAD Z 1 ACC 1
200. LOAD 3 CV

201. MEMBER LOAD
202. 92 TO 94 104 107 109 110 123 TO 125 127 TO 132 139 TO 147 402 TO 404 406 407 -
203. 409 410 412 TO 428 448 TO 463 466 467 469 471 UNI GY -0.75
204. LOAD 4 CM
205. SELFWEIGHT Y -1
206. MEMBER LOAD
207. 92 TO 94 104 107 109 110 123 TO 125 127 TO 132 139 TO 147 402 TO 404 406 407 -
208. 409 410 412 TO 428 448 TO 463 466 467 469 471 UNI GY -1.2
209. *COEFICIENTE DE CAPACIDAD DE DISIPACION DE ENERGIA R= 4.5
210. *COMBINACIONES PARA DISEÑO DE LA ESTRUCTURA Y SUPERESTRUCTURA
211. *COMBINACIONES PARA DISEÑO DE VIGAS Y COLUMNAS
212. LOAD COMB 5 1.2 CM + 1.600 CV
213. 4 1.2 3 1.6
214. LOAD COMB 6 1.2 CM + CV + (SX/R + 0,30 SZ/R)
215. 4 1.2 3 1.0 1 0.222 2 0.067
216. LOAD COMB 7 1.2 CM + CV - (SX/R - 0,30 SZ/R)
217. 4 1.2 3 1.0 1 -0.222 2 0.067
218. LOAD COMB 8 1.2 CM + CV + (SX/R - 0,30 SZ/R)
219. 4 1.2 3 1.0 1 0.222 2 -0.067
220. LOAD COMB 9 1.2 CM + CV - (SX/R + 0,30 SZ/R)
221. 4 1.2 3 1.0 1 -0.222 2 -0.067
222. LOAD COMB 10 1.2 CM + CV + (0.3 SX/R + SZ/R)
223. 4 1.2 3 1.0 1 0.067 2 0.222
224. LOAD COMB 11 1.2 CM + CV - (0.3 SX/R - SZ/R)
225. 4 1.2 3 1.0 1 -0.067 2 0.222
226. LOAD COMB 12 1.2 CM + CV + (0.3 SX/R - SZ/R)
227. 4 1.2 3 1.0 1 0.067 2 -0.222
228. LOAD COMB 13 1.2 CM + CV - (0.3 SX/R + SZ/R)
229. 4 1.2 3 1.0 1 -0.067 2 -0.222
230. LOAD COMB 14 0.9 CM + (SX/R + 0,30 SZ/R)
231. 4 0.9 1 0.222 2 0.067
232. LOAD COMB 15 0.9 CM - (SX/R - 0,30 SZ/R)
233. 4 0.9 1 -0.222 2 0.067
234. LOAD COMB 16 0.9 CM + (SX/R - 0,30 SZ/R)
235. 4 0.9 1 0.222 2 -0.067
236. LOAD COMB 17 0.9 CM - (SX/R + 0,30 SZ/R)
237. 4 0.9 1 -0.222 2 -0.067
238. LOAD COMB 18 0.9 CM + (0.3 SX/R + SZ/R)
239. 4 0.9 1 0.067 2 0.222
240. LOAD COMB 19 0.9 CM - (0.3 SX/R - SZ/R)
241. 4 0.9 1 -0.067 2 0.222
242. LOAD COMB 20 0.9 CM + (0.3 SX/R - SZ/R)
243. 4 0.9 1 0.067 2 -0.222
244. LOAD COMB 21 0.9 CM - (0.3 SX/R + SZ/R)
245. 4 0.9 1 -0.067 2 -0.222
246. *COMBINACIONES PARA DISEÑO DE CIMENTACION
247. *COMBINACIONES DE SERVICIO
248. LOAD COMB 22 CM + CV
249. 4 1.0 3 1.0
250. LOAD COMB 23 CM + 0,75 CV + 0.75 (0.7)(SX + 0,3 SZ)/R
251. 4 1.0 3 0.75 1 0.117 2 0.035
252. LOAD COMB 24 CM + 0,75 CV - 0.75 (0.7)(SX - 0,3 SZ)/R
253. 4 1.0 3 0.75 1 -0.117 2 0.035
254. LOAD COMB 25 CM + 0,75 CV + 0.75 (0.7)(SX - 0,3 SZ)/R
255. 4 1.0 3 0.75 1 0.117 2 -0.035
256. LOAD COMB 26 CM + 0,75 CV - 0.75 (0.7)(SX + 0,3 SZ)/R

257. 4 1.0 3 0.75 1 -0.117 2 -0.035
 258. LOAD COMB 27 CM + 0,75 CV + 0.75 (0.7)(0,3 SX + SZ)/R
 259. 4 1.0 3 0.75 1 0.035 2 0.13
 260. LOAD COMB 28 CM + 0,75 CV - 0.75 (0.7)(0.3 SX - SZ)/R
 261. 4 1.0 3 0.75 1 -0.035 2 0.117
 262. LOAD COMB 29 CM + 0,75 CV + 0.75 (0.7)(0.3 SX - SZ)/R
 263. 4 1.0 3 0.75 1 0.035 2 -0.117
 264. LOAD COMB 30 CM + 0,75 CV - 0.75 (0.7)(0.3 SX + SZ)/R
 265. 4 1.0 3 0.75 1 -0.035 2 -0.117
 266. LOAD COMB 31 0.6 CM + 0.700 (SX + 0.3 SZ)/R
 267. 4 0.6 1 0.156 2 0.047
 268. LOAD COMB 32 0.6 CM - 0.700 (SX - 0.3 SZ)/R
 269. 4 0.6 1 -0.156 2 0.047
 270. LOAD COMB 33 0.6 CM + 0.700 (SX - 0.3 SZ)/R
 271. 4 0.6 1 0.156 2 -0.047
 272. LOAD COMB 34 0.6 CM - 0.700 (SX + 0.3 SZ)/R
 273. 4 0.6 1 -0.156 2 -0.047
 274. LOAD COMB 35 0.6 CM + 0.700 (0.3 SX + SZ)/R
 275. 4 0.6 1 0.047 2 0.156
 276. LOAD COMB 36 0.6 CM - 0.700 (0.3 SX - SZ)/R
 277. 4 0.6 1 -0.047 2 0.156
 278. LOAD COMB 37 0.6 CM - 0.700 (0.3 SX + SZ)/R
 279. 4 0.6 1 -0.047 2 -0.156
 280. LOAD COMB 38 0.6 CM + 0.700 (0.3 SX - SZ)/R
 281. 4 0.6 1 0.047 2 -0.156
 282. PERFORM ANALYSIS

P R O B L E M S T A T I S T I C S

NUMBER OF JOINTS	204	NUMBER OF MEMBERS	349
NUMBER OF PLATES	0	NUMBER OF SOLIDS	0
NUMBER OF SURFACES	0	NUMBER OF SUPPORTS	33

SOLVER USED IS THE OUT-OF-CORE BASIC SOLVER

ORIGINAL/FINAL BAND-WIDTH= 200/ 24/ 126 DOF
 TOTAL PRIMARY LOAD CASES = 4, TOTAL DEGREES OF FREEDOM = 1026
 TOTAL LOAD COMBINATION CASES = 34 SO FAR.
 SIZE OF STIFFNESS MATRIX = 130 DOUBLE KILO-WORDS
 REQD/AVAIL. DISK SPACE = 14.0/ 284753.8 MB

****WARNING: IF THIS UBC/IBC ANALYSIS HAS TENSION/COMPRESSION OR REPEAT LOAD OR RE-ANALYSIS OR SELECT OPTIMIZE, THEN EACH UBC/IBC CASE SHOULD BE FOLLOWED BY PERFORM ANALYSIS _CHANGE.**

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*****
*
* COLOMBIAN SEISMIC LOAD :
*
* TIME PERIODS FOR X DIRECTION:
* Ta = 0.285 Tb = 0.258 Tuser = 0.000
* TIME PERIOD USED (T) = 0.285
* LOAD FACTOR = 1.000
* DESIGN BASE SHEAR = 0.485 X 1180.88 = 573.28 KN
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*
* COLOMBIAN SEISMIC LOAD :
*
* TIME PERIODS FOR Z DIRECTION:
* Ta = 0.285 Tb = 0.214 Tuser = 0.000
* TIME PERIOD USED (T) = 0.285
* LOAD FACTOR = 1.000
* DESIGN BASE SHEAR = 0.485 X 1180.88 = 573.28 KN
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- 283. LOAD LIST 1 2
- 284. PRINT STORY DRIFT

STORY	HEIGHT (METE)	LOAD	DRIFT(CM)		ECCENTRICITY (METE)	RATIO
			X	Z		
BASE=	0.00					
1	0.00	1	0.0000	0.0000	0.0000	L / 999999
		2	0.0000	0.0000	0.0000	L / 999999
2	1.00	1	0.0574	-0.0010	0.0000	L / 1741
		2	-0.0012	0.0614	0.0000	L / 1628
3	3.59	1	0.8027	0.0124	0.0000	L / 447
		2	0.4644	1.0621	0.0000	L / 338
4	3.63	1	0.7879	0.0128	0.0000	L / 461
		2	0.3167	1.0776	0.0000	L / 337
5	3.70	1	0.7139	-0.0003	0.0000	L / 518
		2	0.0211	1.0372	0.0000	L / 357
6	3.80	1	1.0803	-0.0077	0.0000	L / 352
		2	-0.0149	1.0758	0.0000	L / 353
7	3.90	1	1.5477	0.0015	0.0000	L / 252
		2	-0.0321	1.0764	0.0000	L / 362
8	3.98	1	2.0153	-0.0128	0.0000	L / 197
		2	-0.0881	1.0487	0.0000	L / 379
9	4.00	1	1.3605	-0.0026	0.0000	L / 294

DXF IMPORT OF MODELO 3D FALTANTE.DXF				-- PAGE NO.		9
		2	0.0436	1.1165	0.0000	L / 358
10	4.03	1	2.3958	0.0691	0.0000	L / 168
		2	0.0094	0.7566	0.0000	L / 533
11	4.06	1	1.5141	-0.0698	0.0000	L / 268
		2	-0.1083	1.2252	0.0000	L / 331
12	4.10	1	1.5927	0.0045	0.0000	L / 257
		2	0.0682	1.0870	0.0000	L / 377
13	4.16	1	1.6770	-0.0145	0.0000	L / 248
		2	0.0242	1.0942	0.0000	L / 380
14	4.20	1	1.3302	-0.0237	0.0000	L / 316
		2	-0.0690	1.1573	0.0000	L / 363
15	4.27	1	1.4349	-0.0091	0.0000	L / 297
		2	-0.0008	1.1377	0.0000	L / 375
16	4.30	1	0.9584	0.0102	0.0000	L / 448
		2	0.0531	1.0778	0.0000	L / 399
17	4.33	1	1.8970	-0.0199	0.0000	L / 228
		2	-0.0682	1.2165	0.0000	L / 356
18	4.40	1	1.8516	-0.0114	0.0000	L / 238
		2	-0.0174	1.1460	0.0000	L / 384
19	4.50	1	2.1709	-0.0137	0.0000	L / 207
		2	-0.0287	1.1686	0.0000	L / 385
20	4.60	1	2.4991	-0.0114	0.0000	L / 184
		2	-0.0061	1.1413	0.0000	L / 403
21	4.70	1	1.9457	0.0040	0.0000	L / 241
		2	-0.0466	1.0778	0.0000	L / 436
22	4.80	1	1.9984	-0.0116	0.0000	L / 240
		2	-0.0038	1.1373	0.0000	L / 422
23	4.90	1	1.3617	-0.0030	0.0000	L / 360

STORY	HEIGHT (METE)	LOAD	DRIFT(CM)		ECCENTRICITY (METE)	RATIO
			X	Z		
BASE=	0.00					
		2	-0.0525	1.1149	0.0000	L / 439
24	5.00	1	1.8118	-0.0119	0.0000	L / 276
		2	0.2454	1.1694	0.0000	L / 428
285. LOAD LIST 5 TO 21						
286. START CONCRETE DESIGN						
287. CODE ACI						
288. CLB 0.04 MEMB 9 TO 25 30 31 42 TO 44 54 TO 58 67 TO 70 101 472						
289. CLS 0.04 MEMB 9 TO 25 30 31 42 TO 44 54 TO 58 67 TO 70 101 472						
290. CLT 0.04 MEMB 9 TO 25 30 31 42 TO 44 54 TO 58 67 TO 70 101 472						
291. FC 21000 MEMB 9 TO 25 30 31 42 TO 44 54 TO 58 67 TO 70 101 472						
292. FYMAIN 420000 MEMB 9 TO 25 30 31 42 TO 44 54 TO 58 67 TO 70 101 472						
293. FYSEC 420000 MEMB 9 TO 25 30 31 42 TO 44 54 TO 58 67 TO 70 101 472						
294. DESIGN COLUMN 9 TO 25 30 31 42 TO 44 54 TO 58 67 TO 70 101 472						

=====

COLUMN NO. 9 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 10 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 11 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 12 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 13 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 14 DESIGN PER ACI 318-08 - AXIAL + BENDING

DXF IMPORT OF MODELO 3D FALTANTE.DXF

-- PAGE NO. 13

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
 ONLY MINIMUM STEEL IS REQUIRED.
 AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
 TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 15 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
 ONLY MINIMUM STEEL IS REQUIRED.
 AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
 TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 16 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
 ONLY MINIMUM STEEL IS REQUIRED.
 AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
 TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 17 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 18 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 19 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 20 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 21 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 22 DESIGN PER ACI 318-08 - AXIAL + BENDING

DXF IMPORT OF MODELO 3D FALTANTE.DXF

-- PAGE NO. 16

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
 ONLY MINIMUM STEEL IS REQUIRED.
 AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
 TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 23 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
 ONLY MINIMUM STEEL IS REQUIRED.
 AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
 TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 24 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
 ONLY MINIMUM STEEL IS REQUIRED.
 AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
 TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 25 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 30 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 31 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 42 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 43 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 44 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
 ONLY MINIMUM STEEL IS REQUIRED.
 AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
 TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 54 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
 ONLY MINIMUM STEEL IS REQUIRED.
 AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
 TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 55 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
 ONLY MINIMUM STEEL IS REQUIRED.
 AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
 TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 56 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 57 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 58 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 67 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 68 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 69 DESIGN PER ACI 318-08 - AXIAL + BENDING

DXF IMPORT OF MODELO 3D FALTANTE.DXF

-- PAGE NO. 22

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
 ONLY MINIMUM STEEL IS REQUIRED.
 AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
 TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 70 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
 ONLY MINIMUM STEEL IS REQUIRED.
 AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
 TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 101 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
 ONLY MINIMUM STEEL IS REQUIRED.
 AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION	REINF PCT.	LOAD	LOCATION	PHI
4 - 16 MM	1.287	5	END	0.650

(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
 TIE BAR NUMBER 12 SPACING 250.00 MM

=====

COLUMN NO. 472 DESIGN PER ACI 318-08 - AXIAL + BENDING

FY - 420.0 FC - 21.0 MPA, SQRE SIZE - 250.0 X 250.0 MMS, TIED
ONLY MINIMUM STEEL IS REQUIRED.
AREA OF STEEL REQUIRED = 625.0 SQ. MM

BAR CONFIGURATION REINF PCT. LOAD LOCATION PHI

4 - 16 MM 1.287 5 END 0.650
(PROVIDE EQUAL NUMBER OF BARS ON EACH FACE)
TIE BAR NUMBER 12 SPACING 250.00 MM

*****END OF COLUMN DESIGN RESULTS*****

295. END CONCRETE DESIGN
296. FINISH

***** END OF THE STAAD.Pro RUN *****

**** DATE= DEC 22,2014 TIME= 18:47:53 ****

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*****
*   For technical assistance on STAAD.Pro, please visit   *
*   http://selectservices.bentley.com/en-US/             *
*                                                         *
*   Details about additional assistance from               *
*   Bentley and Partners can be found at program menu    *
*   Help->Technical Support                               *
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